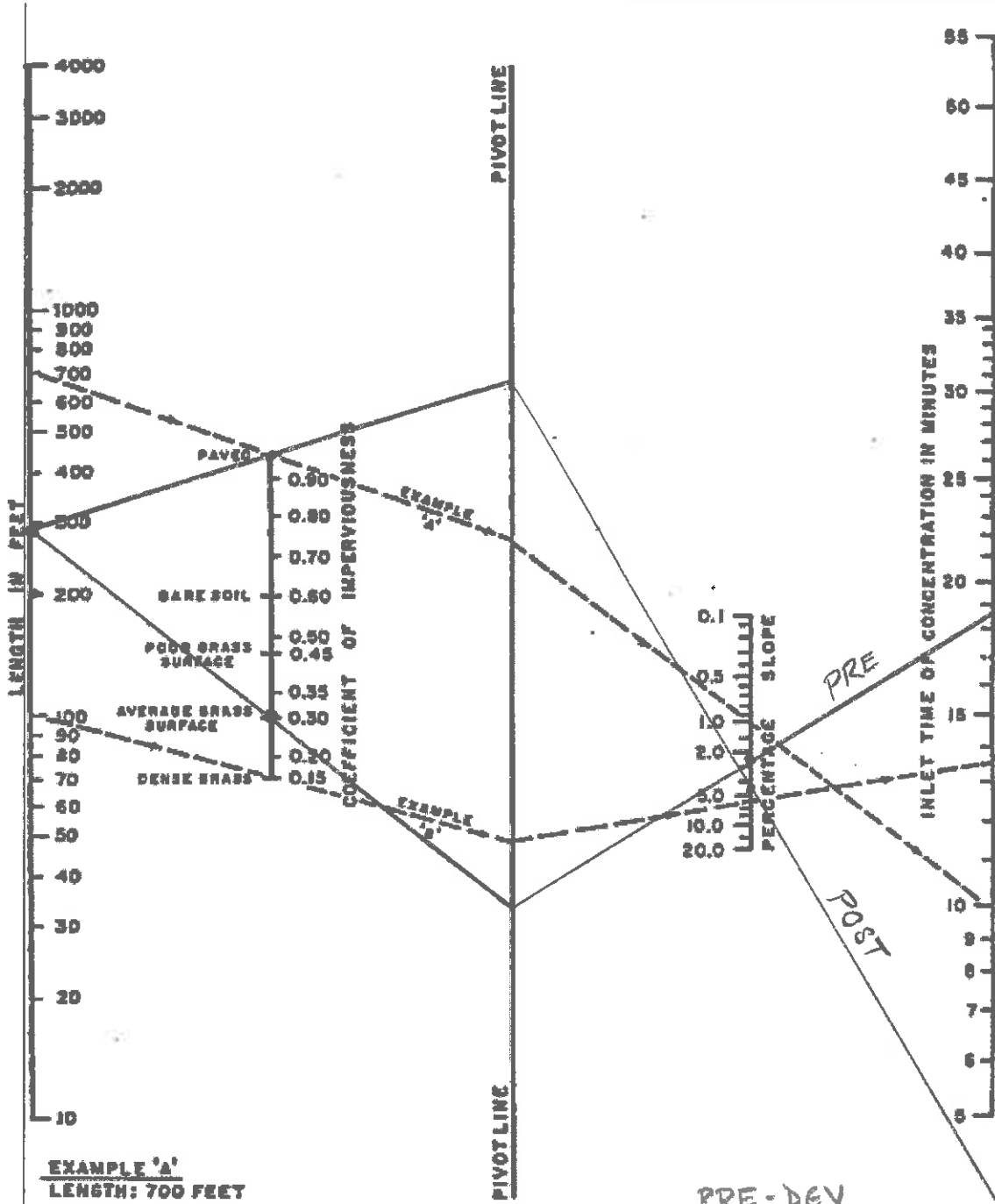


APPENDIX D, TABLE D-4



**EXAMPLE 'A'**  
 LENGTH: 700 FEET  
 PAVED  
 SLOPE: 1.0 %  
 TIME: 10 MINUTES

**EXAMPLE 'B'**  
 LENGTH: 100 FEET  
 DENSE GRASS  
 SLOPE: 6.0 %  
 TIME: 13 1/2 MINUTES

SEELYE CHART  
 TIME OF CONCENTRATION

PRE-DEV  
 291'  
 GRASS  
 ~ 2.5 %  
 $T_c \sim 19 \text{ min}$

POST-DEV  
 301'  
 PAVED  
 ~ 4.5 %  
 $T_c \sim 5 \text{ min}$

LOT AREA (USE AS DRAINAGE AREA) = 39259 ft<sup>2</sup>  
(0.901 Ac)

17 110  
Skyline Ross  
9/8/17

EX slope ~ 2.5% 291' C ~ .3  
From Table D-4 T<sub>PRE</sub> ~ 19 min

PR. slope ~ 4.0% 201' C ~ .9  
T<sub>POST</sub> ~ 5 min (10 minute minimum)

Teresa Barnes  
barnest@bceo.org

area green = 5779 ft<sup>2</sup>

→ impervious = 39259 - 5779 = 33480 ft<sup>2</sup> (85% impervious)

$$C_{POST} = \frac{(.3 \times 5779 + .9 \times 33480)}{39259} = 0.81$$

1 year 24 hour depth ~ 2.4" (2 year = 2.8" which matches Greene County)

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S} \quad (\text{inches})$$

$$S = \frac{1000}{CN} - 10 \quad \begin{array}{l} CN_{PRE} = 74 \\ CN_{POST} = 98 \end{array}$$

$$Q_{PRE} = \frac{(2.4 - 0.2 * 3.5135)^2}{2.4 + 0.8 * 3.5135} = 0.55''$$

$$S_{PRE} = \frac{1000}{74} - 10 = 3.5135$$

$$S_{POST} = \frac{1000}{98} - 10 = 0.2041$$

$$Q_{POST} = \frac{(2.4 - 0.2 * 0.2041)^2}{2.4 + 0.8 * 0.2041} = 2.17''$$

$$V_{PRE} = 0.55 \text{ inches} \cdot \frac{ft}{12''} \cdot 39259 \text{ ft}^2 = 1799 \text{ ft}^3$$

$$V_{POST} = 2.17 \cdot \frac{1}{12} \cdot 39259 = 7099 \text{ ft}^3$$

$$\% \text{ INC} = \frac{7099 - 1799}{1799} \times 100 = 295\%$$

↳ CRITICAL STORM = 50 year

**Water Quality Calculation-**

**Skyline Ross Twp**

Post Developed Properties	Area (sf)	Area (acres)
Total	39259	0.90
Pervious	5779	0.13
Impervious	33480	0.77

$$WQv \text{ "C"} = 0.858i^3 - 0.78i^2 + 0.774i + 0.04$$

where i = impervious area ratio      0.8528

Total Area=                              0.9013

WQv C =                                      0.6649

**Water Quality Volume (WQv) Required**

Precipitation	0.75	in
Area, A	0.9013	acres
WQv req=WQv C * 0.75 * A/12	0.0375	acre-feet
	1631.55	cubic feet
Add 20% for sedimentation	<b>1958</b>	<b>cubic feet</b>

<b>Orifice Sizing</b>	Flow Rate (Q)=	0.011	cfs
Using Extended Detention with	volume/48 hr*1 hr/60min *1 min/60 sec		
48 hour draw down			
Q=0.6 *A * sqrt (2 g h)			
solve for area, convert to orifice diameter			
where h= elevation at WQv minus invert			
WQv elevation=	563.25		
orifice invert elevation=	561.15		
head(h)=	2.1	ft	
A=	0.0016	ft	
Radius	0.0227	ft	
Diameter =	<b>0.546</b>	<b>in</b>	

**Project: Skyline Ross Twp**



Chamber Model -  
 Units -  
 Number of chambers -  
 Voids in the stone (porosity) -  
 Base of Stone Elevation -  
 Amount of Stone Above Chambers -  
 Amount of Stone Below Chambers -  
 Area of system -

SC-740
Imperial <a href="#">Click Here for Metric</a>
4
30
40 %
561.70 ft
6 in <input checked="" type="checkbox"/> Include Perimeter Stone in Calculations
6 in
1400 sf Min. Area - 1014 sf min. area

**StormTech SC-740 Cumulative Storage Volumes**

Height of System (inches)	Incremental Single Chamber (cubic feet)	Incremental Total Chamber (cubic feet)	Incremental Stone (cubic feet)	Incremental Ch & St (cubic feet)	Cumulative Chamber (cubic feet)	Elevation (feet)
42	0.00	0.00	46.67	46.67	2787.12	565.20
41	0.00	0.00	46.67	46.67	2740.45	565.12
40	0.00	0.00	46.67	46.67	2693.78	565.03
39	0.00	0.00	46.67	46.67	2647.12	564.95
38	0.00	0.00	46.67	46.67	2600.45	564.87
37	0.00	0.00	46.67	46.67	2553.78	564.78
36	0.05	1.65	46.01	47.66	2507.12	564.70
35	0.16	4.89	44.71	49.60	2459.46	564.62
34	0.28	8.46	43.28	51.74	2409.86	564.53
33	0.60	18.12	39.42	57.54	2358.12	564.45
32	0.80	24.05	37.05	61.10	2300.58	564.37
31	0.95	28.52	35.26	63.78	2239.48	564.28
30	1.07	32.24	33.77	66.01	2175.70	564.20
29	1.18	35.41	32.50	67.92	2109.70	564.12
28	1.27	37.97	31.48	69.45	2041.78	564.03
27	1.36	40.65	30.41	71.06	1972.33	563.95
26	1.45	43.62	29.22	72.84	1901.27	563.87
25	1.52	45.74	28.37	74.11	1828.43	563.78
24	1.58	47.47	27.68	75.15	1754.32	563.70
23	1.64	49.27	26.96	76.23	1679.17	563.62
22	1.70	50.99	26.27	77.26	1602.95	563.53
21	1.75	52.59	25.63	78.22	1525.69	563.45
20	1.80	54.08	25.03	79.12	1447.47	563.37
19	1.85	55.65	24.41	80.06	1368.35	563.28
18	1.89	56.79	23.95	80.74	1288.29	563.20
17	1.93	58.02	23.46	81.48	1207.55	563.12
16	1.97	59.25	22.97	82.22	1126.07	563.03
15	2.01	60.30	22.55	82.85	1043.86	562.95
14	2.04	61.35	22.13	83.48	961.01	562.87
13	2.07	62.25	21.77	84.02	877.54	562.78
12	2.10	63.15	21.41	84.56	793.52	562.70
11	2.13	63.95	21.09	85.04	708.96	562.62
10	2.15	64.62	20.82	85.44	623.93	562.53
9	2.18	65.31	20.54	85.85	538.49	562.45
8	2.20	65.95	20.29	86.24	452.64	562.37
7	2.21	66.22	20.18	86.40	366.40	562.28
6	0.00	0.00	46.67	46.67	280.00	562.20
5	0.00	0.00	46.67	46.67	233.33	562.12
4	0.00	0.00	46.67	46.67	186.67	562.03
3	0.00	0.00	46.67	46.67	140.00	561.95
2	0.00	0.00	46.67	46.67	93.33	561.87
1	0.00	0.00	46.67	46.67	46.67	561.78

# Hydrograph Report

## Hyd. No. 1

Pre-Developed

Hydrograph type	= Rational	Peak discharge	= 0.655 cfs
Storm frequency	= 1 yrs	Time to peak	= 0.32 hrs
Time interval	= 1 min	Hyd. volume	= 747 cuft
Drainage area	= 0.901 ac	Runoff coeff.	= 0.3
Intensity	= 2.424 in/hr	Tc by User	= 19.00 min
IDF Curve	= butler county.IDF	Asc/Rec limb fact	= 1/1

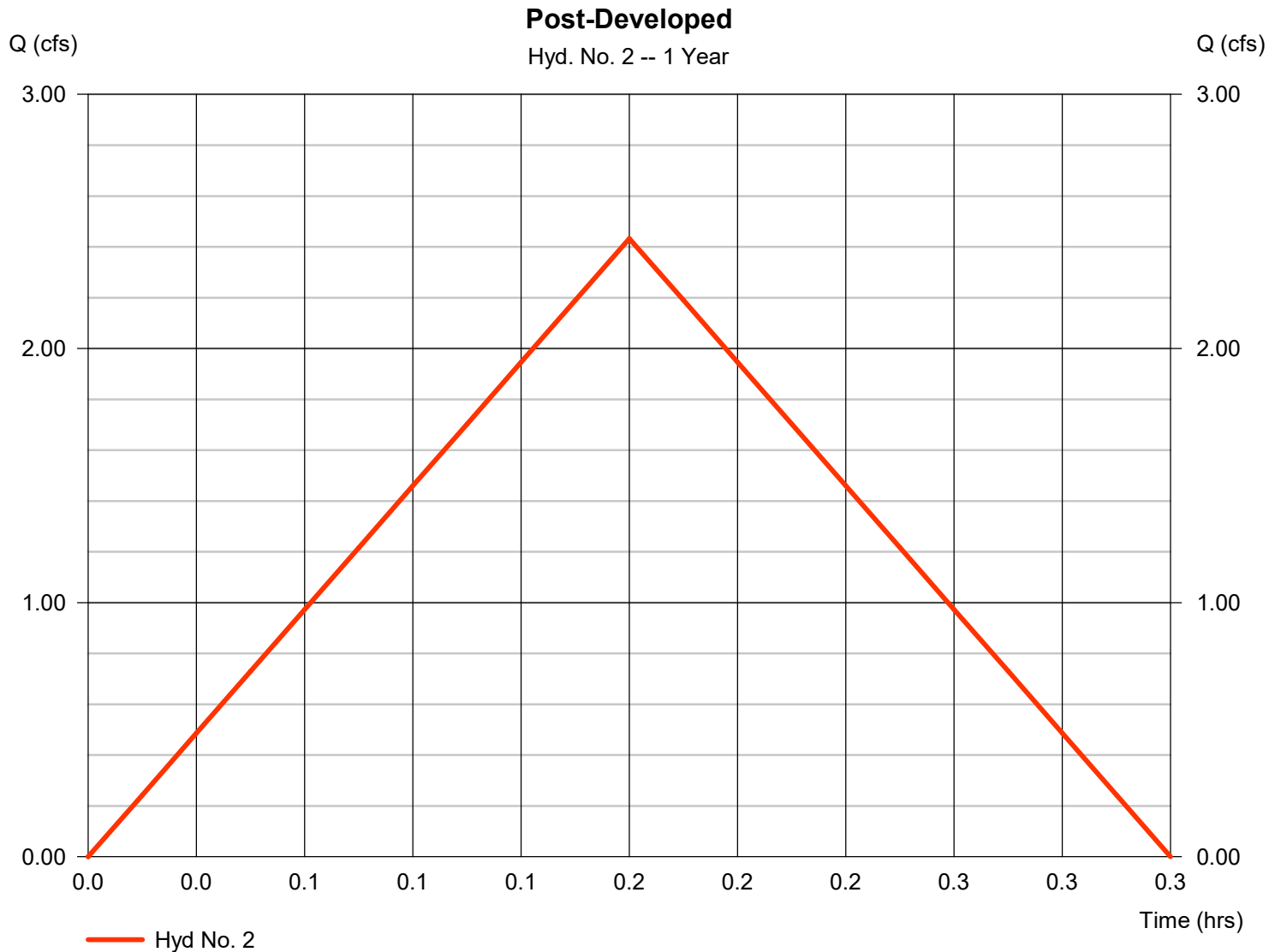


# Hydrograph Report

## Hyd. No. 2

Post-Developed

Hydrograph type	= Rational	Peak discharge	= 2.433 cfs
Storm frequency	= 1 yrs	Time to peak	= 0.17 hrs
Time interval	= 1 min	Hyd. volume	= 1,460 cuft
Drainage area	= 0.901 ac	Runoff coeff.	= 0.81
Intensity	= 3.333 in/hr	Tc by User	= 10.00 min
IDF Curve	= butler county.IDF	Asc/Rec limb fact	= 1/1



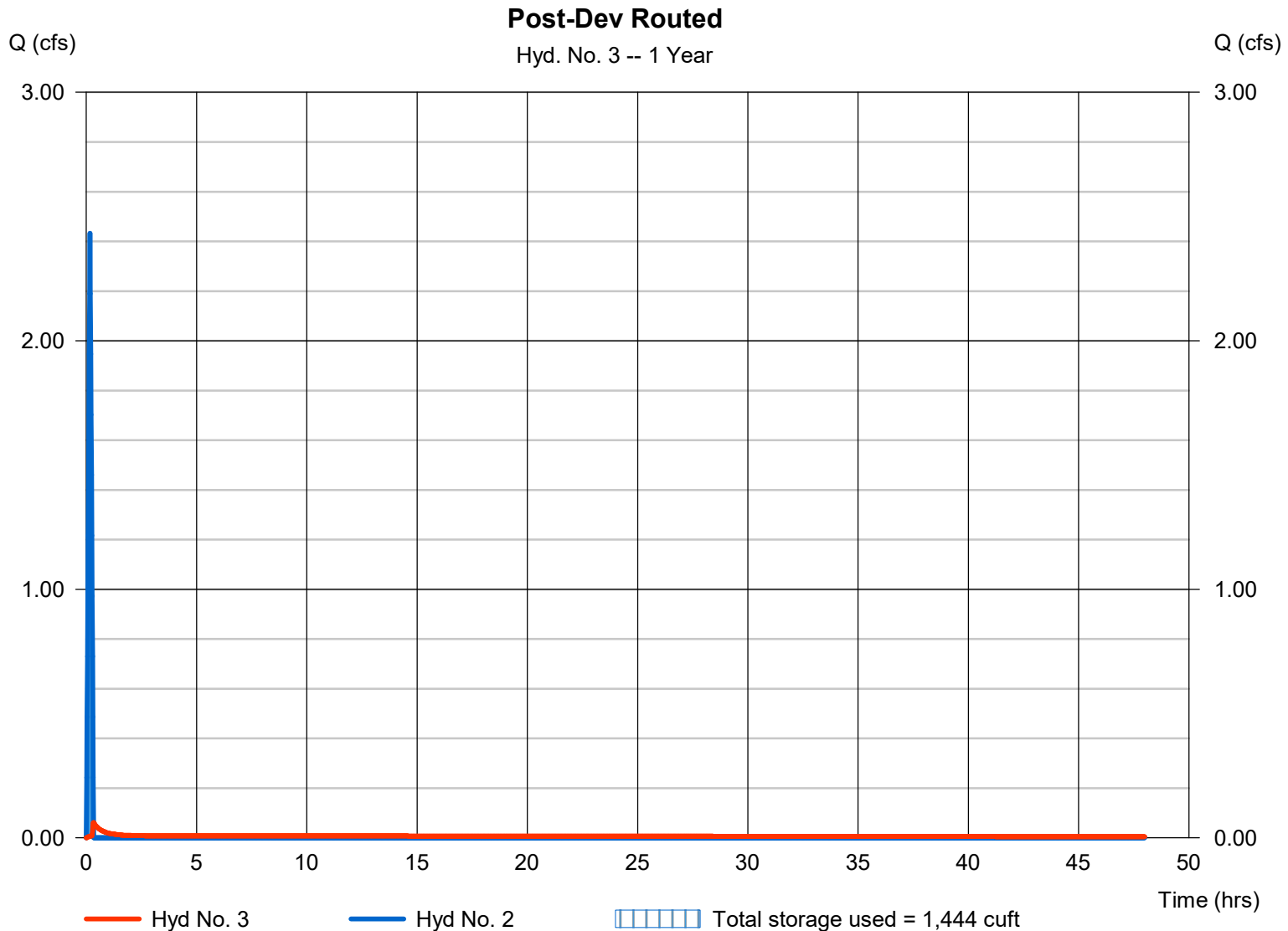
# Hydrograph Report

## Hyd. No. 3

Post-Dev Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.062 cfs
Storm frequency	= 1 yrs	Time to peak	= 0.33 hrs
Time interval	= 1 min	Hyd. volume	= 1,198 cuft
Inflow hyd. No.	= 2 - Post-Developed	Max. Elevation	= 563.40 ft
Reservoir name	= UG Chambers	Max. Storage	= 1,444 cuft

Storage Indication method used.



# Pond Report

## Pond No. 1 - UG Chambers

### Pond Data

**UG Chambers** -Invert elev. = 562.20 ft, Rise x Span = 2.50 x 4.25 ft, Barrel Len = 7.12 ft, No. Barrels = 30, Slope = 0.00%, Headers = No  
**Encasement** -Invert elev. = 561.70 ft, Width = 5.25 ft, Height = 3.50 ft, Voids = 46.85%

### Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	561.70	n/a	0	0
0.35	562.05	n/a	184	184
0.70	562.40	n/a	280	464
1.05	562.75	n/a	351	815
1.40	563.10	n/a	345	1,160
1.75	563.45	n/a	336	1,497
2.10	563.80	n/a	322	1,819
2.45	564.15	n/a	302	2,122
2.80	564.50	n/a	272	2,393
3.15	564.85	n/a	209	2,603
3.50	565.20	n/a	184	2,787

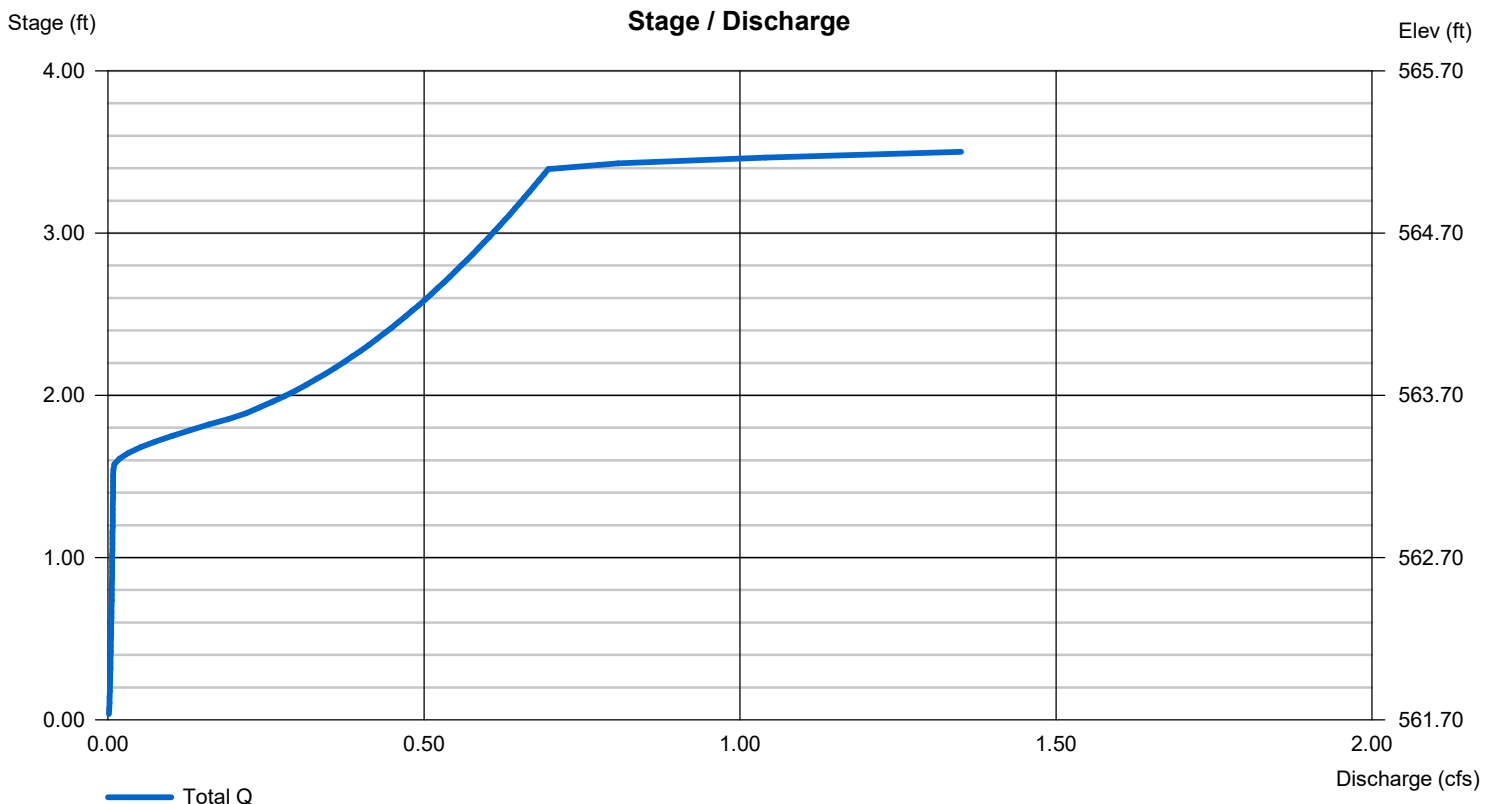
### Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 12.00	0.50	4.50	0.00
Span (in)	= 12.00	0.50	4.50	0.00
No. Barrels	= 1	1	1	0
Invert El. (ft)	= 561.15	561.15	563.25	0.00
Length (ft)	= 10.00	0.50	0.50	0.00
Slope (%)	= 1.00	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	Yes	Yes	No

### Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 6.00	0.00	0.00	0.00
Crest El. (ft)	= 565.10	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	---	---	---
Multi-Stage	= Yes	No	No	No
Exfil.(in/hr)	= 0.000 (by Contour)			
TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).





# Hydrograph Report

## Hyd. No. 1

Pre-Developed

Hydrograph type	= Rational	Peak discharge	= 0.796 cfs
Storm frequency	= 2 yrs	Time to peak	= 0.32 hrs
Time interval	= 1 min	Hyd. volume	= 907 cuft
Drainage area	= 0.901 ac	Runoff coeff.	= 0.3
Intensity	= 2.944 in/hr	Tc by User	= 19.00 min
IDF Curve	= butler county.IDF	Asc/Rec limb fact	= 1/1

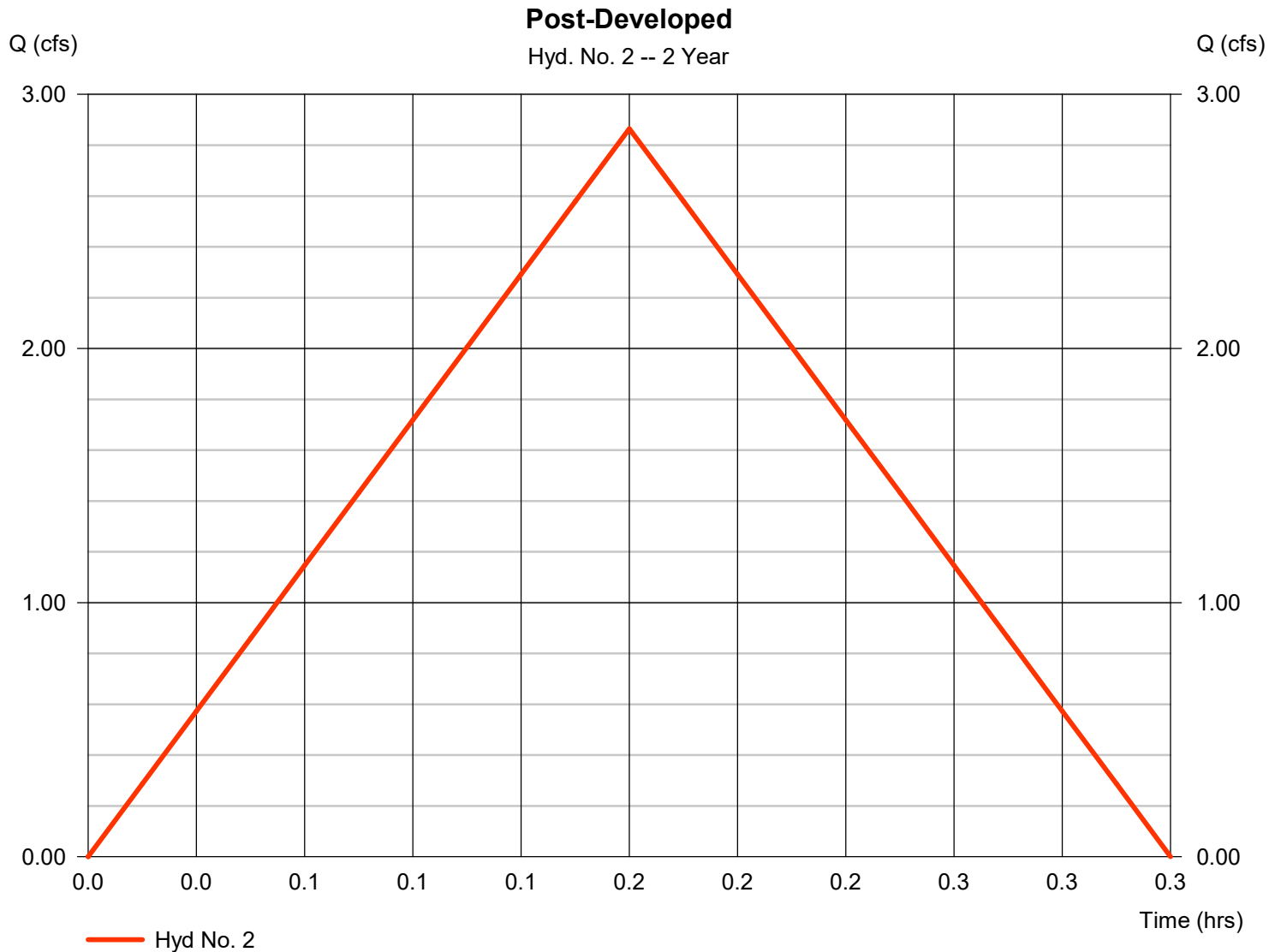


# Hydrograph Report

## Hyd. No. 2

Post-Developed

Hydrograph type	= Rational	Peak discharge	= 2.865 cfs
Storm frequency	= 2 yrs	Time to peak	= 0.17 hrs
Time interval	= 1 min	Hyd. volume	= 1,719 cuft
Drainage area	= 0.901 ac	Runoff coeff.	= 0.81
Intensity	= 3.926 in/hr	Tc by User	= 10.00 min
IDF Curve	= butler county.IDF	Asc/Rec limb fact	= 1/1



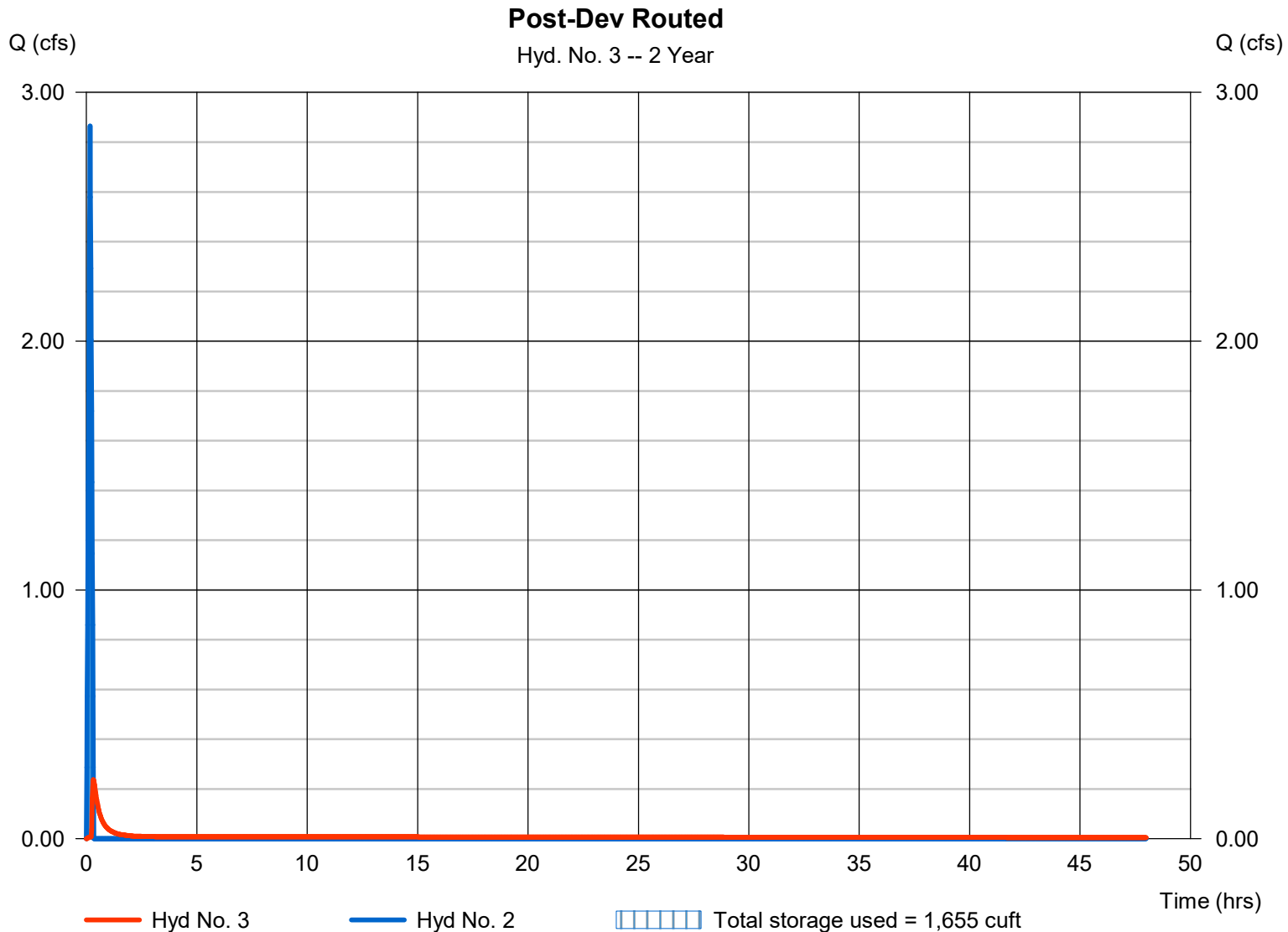
# Hydrograph Report

## Hyd. No. 3

Post-Dev Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.238 cfs
Storm frequency	= 2 yrs	Time to peak	= 0.32 hrs
Time interval	= 1 min	Hyd. volume	= 1,450 cuft
Inflow hyd. No.	= 2 - Post-Developed	Max. Elevation	= 563.62 ft
Reservoir name	= UG Chambers	Max. Storage	= 1,655 cuft

Storage Indication method used.



# Hydrograph Report

## Hyd. No. 1

Pre-Developed

Hydrograph type	= Rational	Peak discharge	= 0.932 cfs
Storm frequency	= 5 yrs	Time to peak	= 0.32 hrs
Time interval	= 1 min	Hyd. volume	= 1,062 cuft
Drainage area	= 0.901 ac	Runoff coeff.	= 0.3
Intensity	= 3.447 in/hr	Tc by User	= 19.00 min
IDF Curve	= butler county.IDF	Asc/Rec limb fact	= 1/1

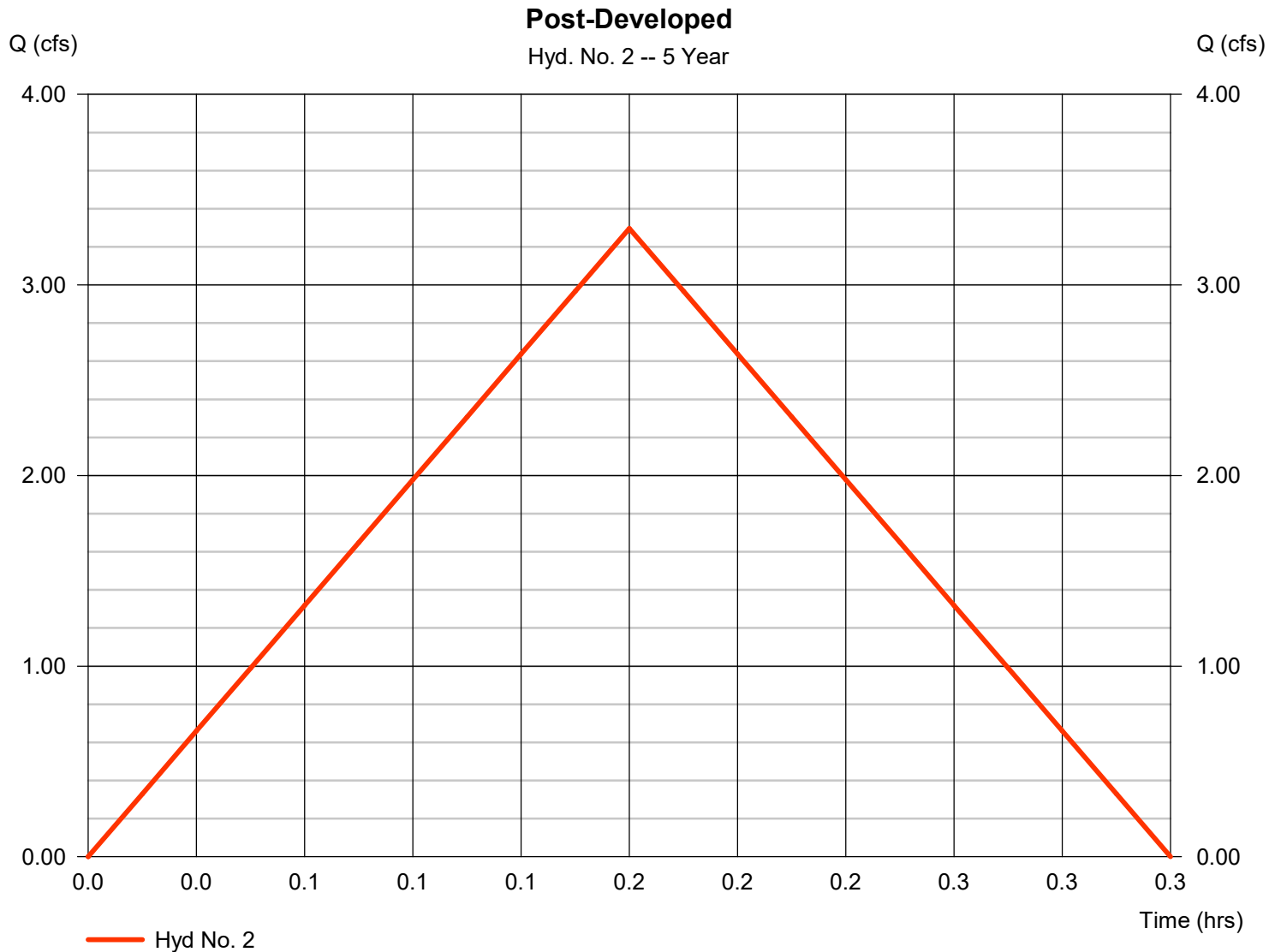


# Hydrograph Report

## Hyd. No. 2

Post-Developed

Hydrograph type	= Rational	Peak discharge	= 3.297 cfs
Storm frequency	= 5 yrs	Time to peak	= 0.17 hrs
Time interval	= 1 min	Hyd. volume	= 1,978 cuft
Drainage area	= 0.901 ac	Runoff coeff.	= 0.81
Intensity	= 4.517 in/hr	Tc by User	= 10.00 min
IDF Curve	= butler county.IDF	Asc/Rec limb fact	= 1/1



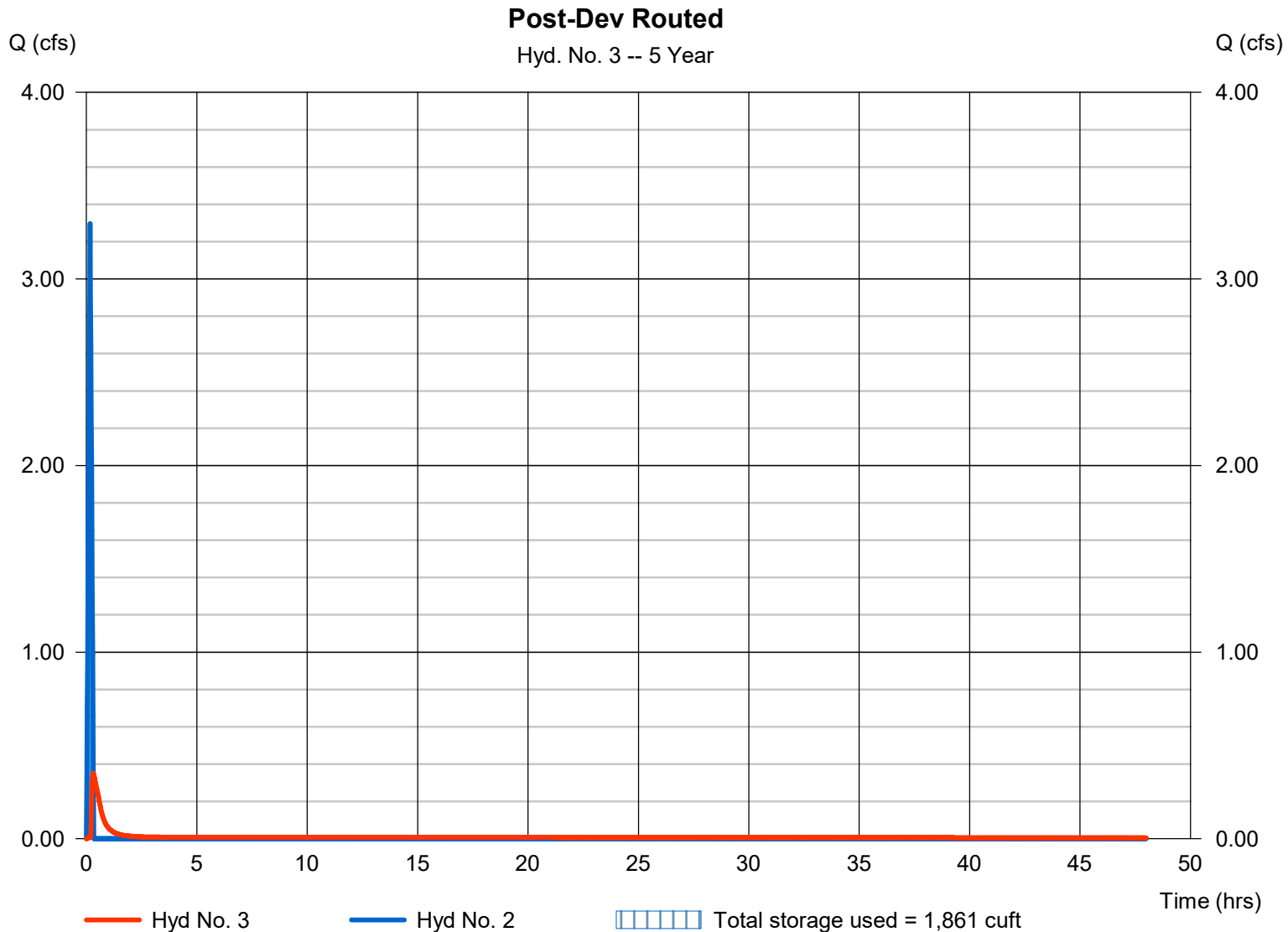
# Hydrograph Report

## Hyd. No. 3

Post-Dev Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.350 cfs
Storm frequency	= 5 yrs	Time to peak	= 0.32 hrs
Time interval	= 1 min	Hyd. volume	= 1,706 cuft
Inflow hyd. No.	= 2 - Post-Developed	Max. Elevation	= 563.85 ft
Reservoir name	= UG Chambers	Max. Storage	= 1,861 cuft

Storage Indication method used.



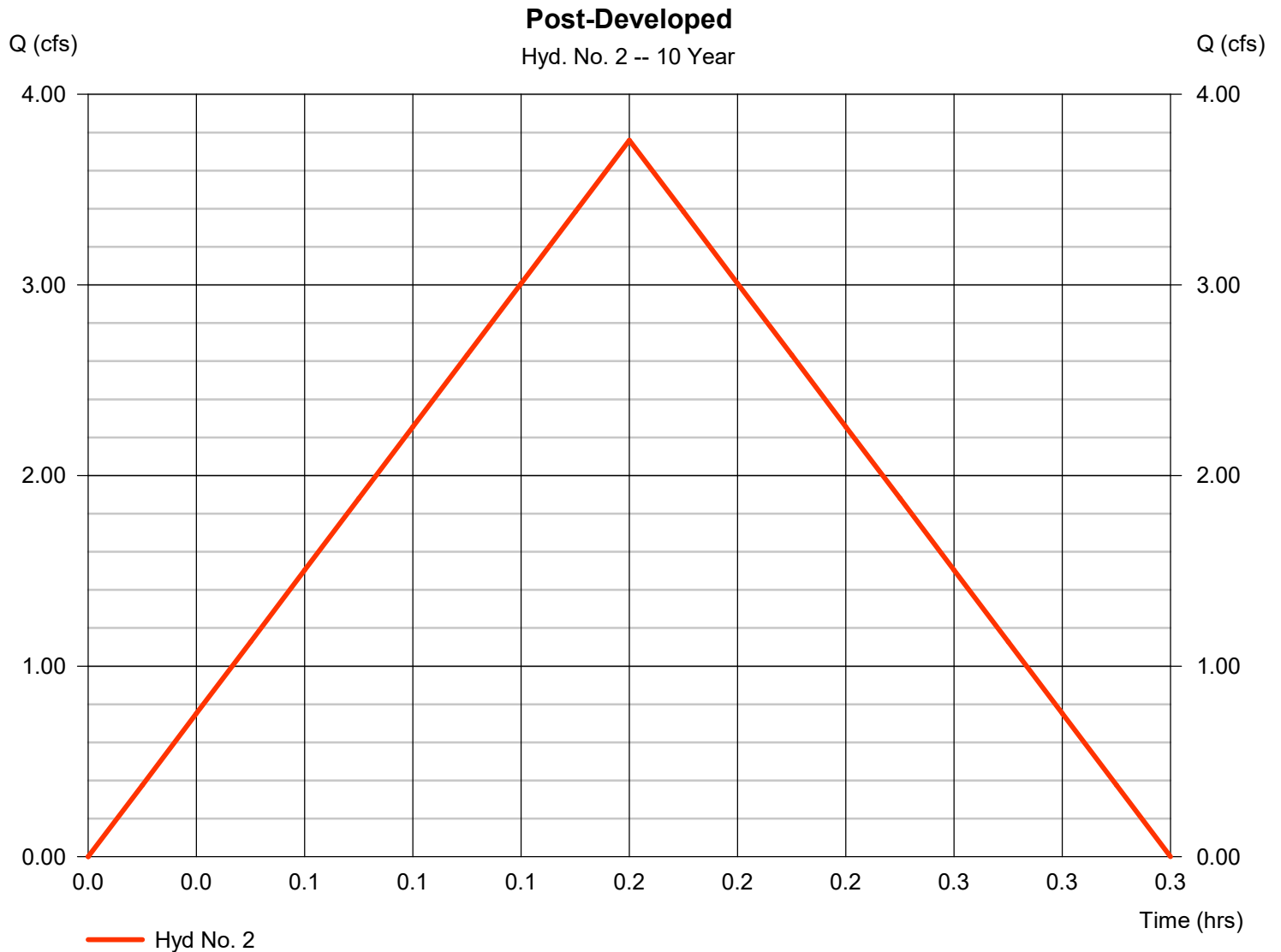


# Hydrograph Report

## Hyd. No. 2

Post-Developed

Hydrograph type	= Rational	Peak discharge	= 3.760 cfs
Storm frequency	= 10 yrs	Time to peak	= 0.17 hrs
Time interval	= 1 min	Hyd. volume	= 2,256 cuft
Drainage area	= 0.901 ac	Runoff coeff.	= 0.81
Intensity	= 5.152 in/hr	Tc by User	= 10.00 min
IDF Curve	= butler county.IDF	Asc/Rec limb fact	= 1/1





# Hydrograph Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

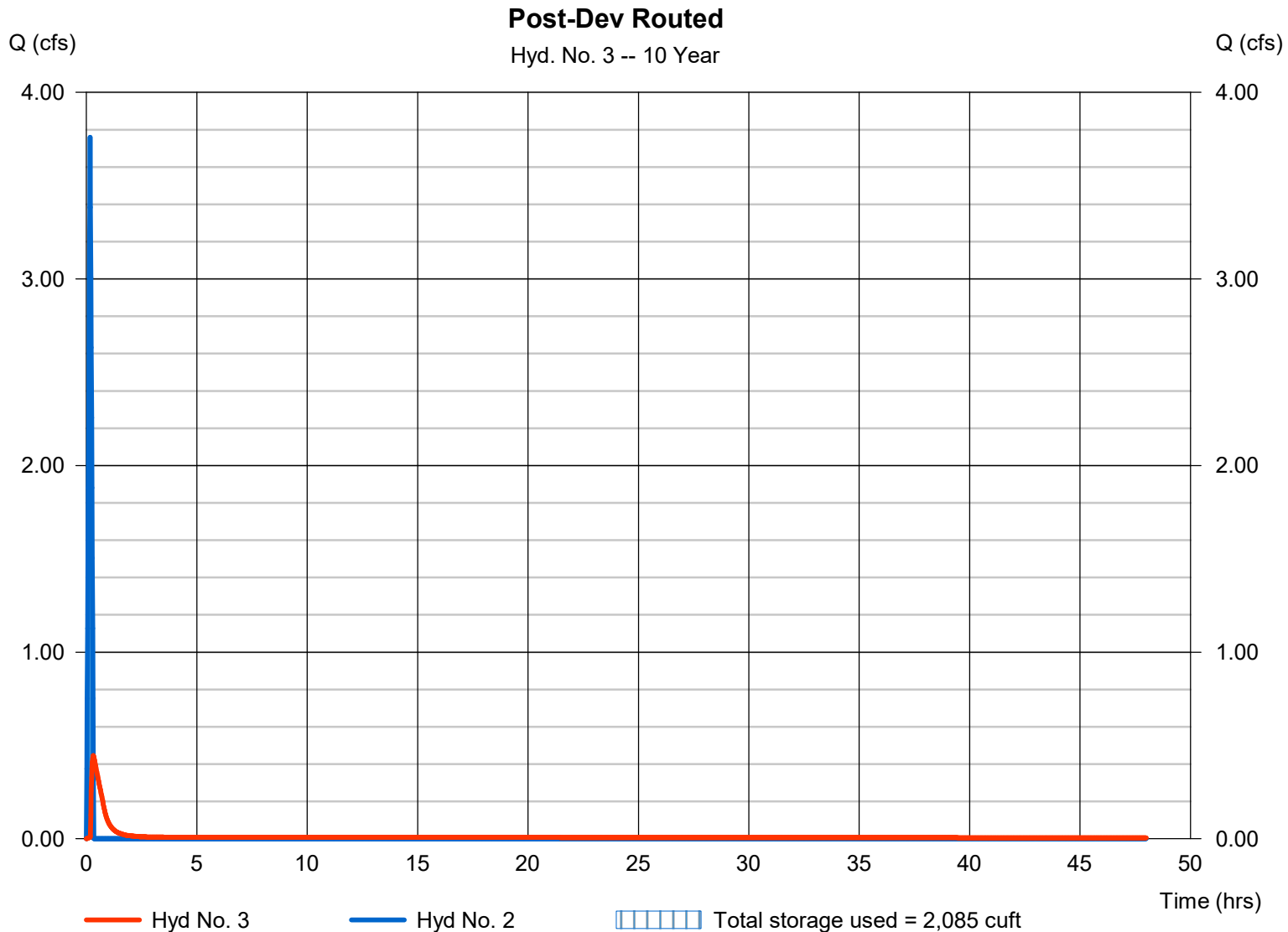
Thursday, 09 / 14 / 2017

## Hyd. No. 3

Post-Dev Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.446 cfs
Storm frequency	= 10 yrs	Time to peak	= 0.32 hrs
Time interval	= 1 min	Hyd. volume	= 1,981 cuft
Inflow hyd. No.	= 2 - Post-Developed	Max. Elevation	= 564.11 ft
Reservoir name	= UG Chambers	Max. Storage	= 2,085 cuft

Storage Indication method used.



# Hydrograph Report

## Hyd. No. 1

Pre-Developed

Hydrograph type	= Rational	Peak discharge	= 1.269 cfs
Storm frequency	= 25 yrs	Time to peak	= 0.32 hrs
Time interval	= 1 min	Hyd. volume	= 1,446 cuft
Drainage area	= 0.901 ac	Runoff coeff.	= 0.3
Intensity	= 4.694 in/hr	Tc by User	= 19.00 min
IDF Curve	= butler county.IDF	Asc/Rec limb fact	= 1/1

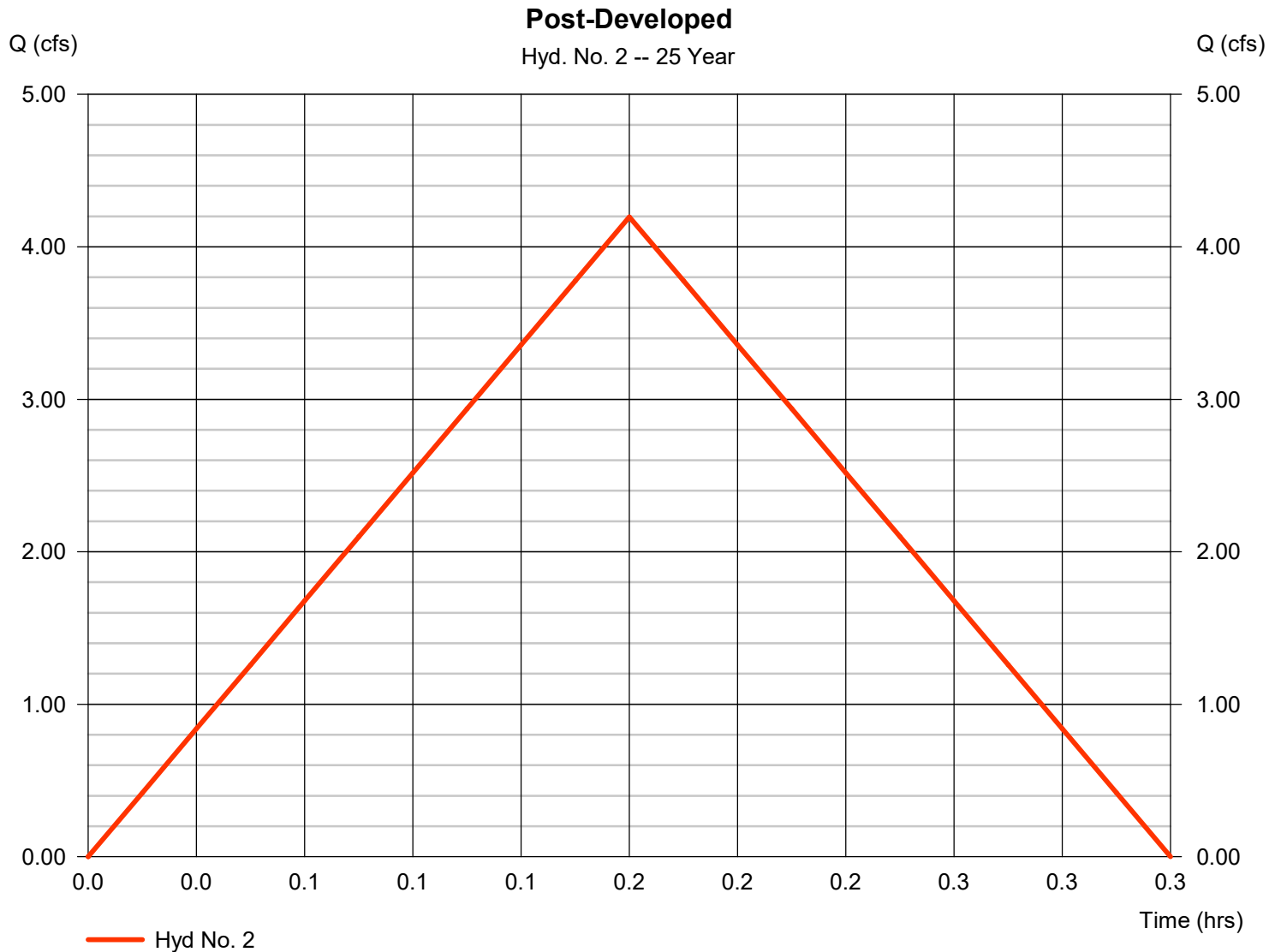


# Hydrograph Report

## Hyd. No. 2

Post-Developed

Hydrograph type	= Rational	Peak discharge	= 4.196 cfs
Storm frequency	= 25 yrs	Time to peak	= 0.17 hrs
Time interval	= 1 min	Hyd. volume	= 2,518 cuft
Drainage area	= 0.901 ac	Runoff coeff.	= 0.81
Intensity	= 5.750 in/hr	Tc by User	= 10.00 min
IDF Curve	= butler county.IDF	Asc/Rec limb fact	= 1/1



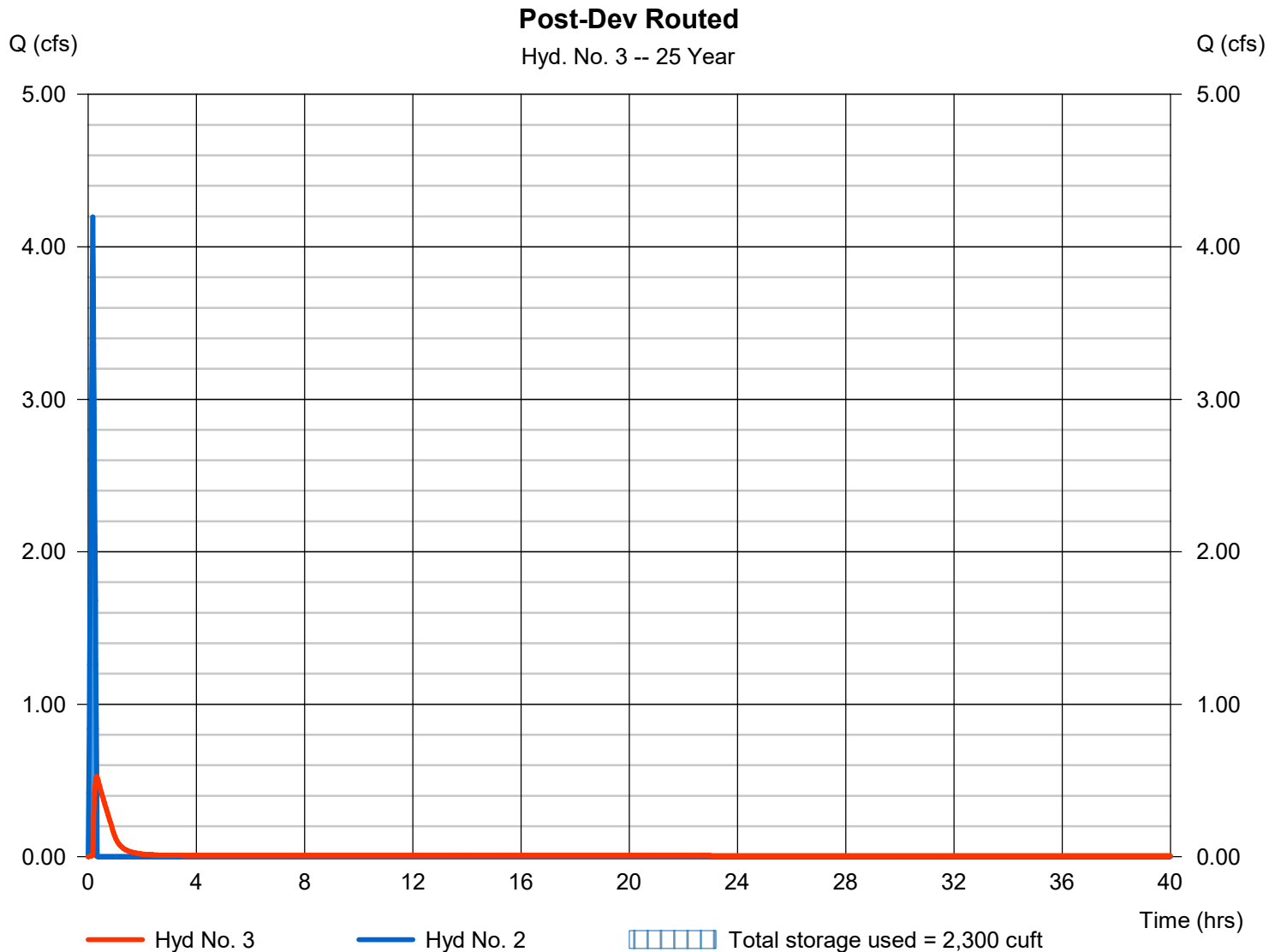
# Hydrograph Report

## Hyd. No. 3

Post-Dev Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.527 cfs
Storm frequency	= 25 yrs	Time to peak	= 0.32 hrs
Time interval	= 1 min	Hyd. volume	= 2,242 cuft
Inflow hyd. No.	= 2 - Post-Developed	Max. Elevation	= 564.38 ft
Reservoir name	= UG Chambers	Max. Storage	= 2,300 cuft

Storage Indication method used.



# Hydrograph Report

## Hyd. No. 1

Pre-Developed

Hydrograph type	= Rational	Peak discharge	= 1.469 cfs
Storm frequency	= 50 yrs	Time to peak	= 0.32 hrs
Time interval	= 1 min	Hyd. volume	= 1,675 cuft
Drainage area	= 0.901 ac	Runoff coeff.	= 0.3
Intensity	= 5.435 in/hr	Tc by User	= 19.00 min
IDF Curve	= butler county.IDF	Asc/Rec limb fact	= 1/1

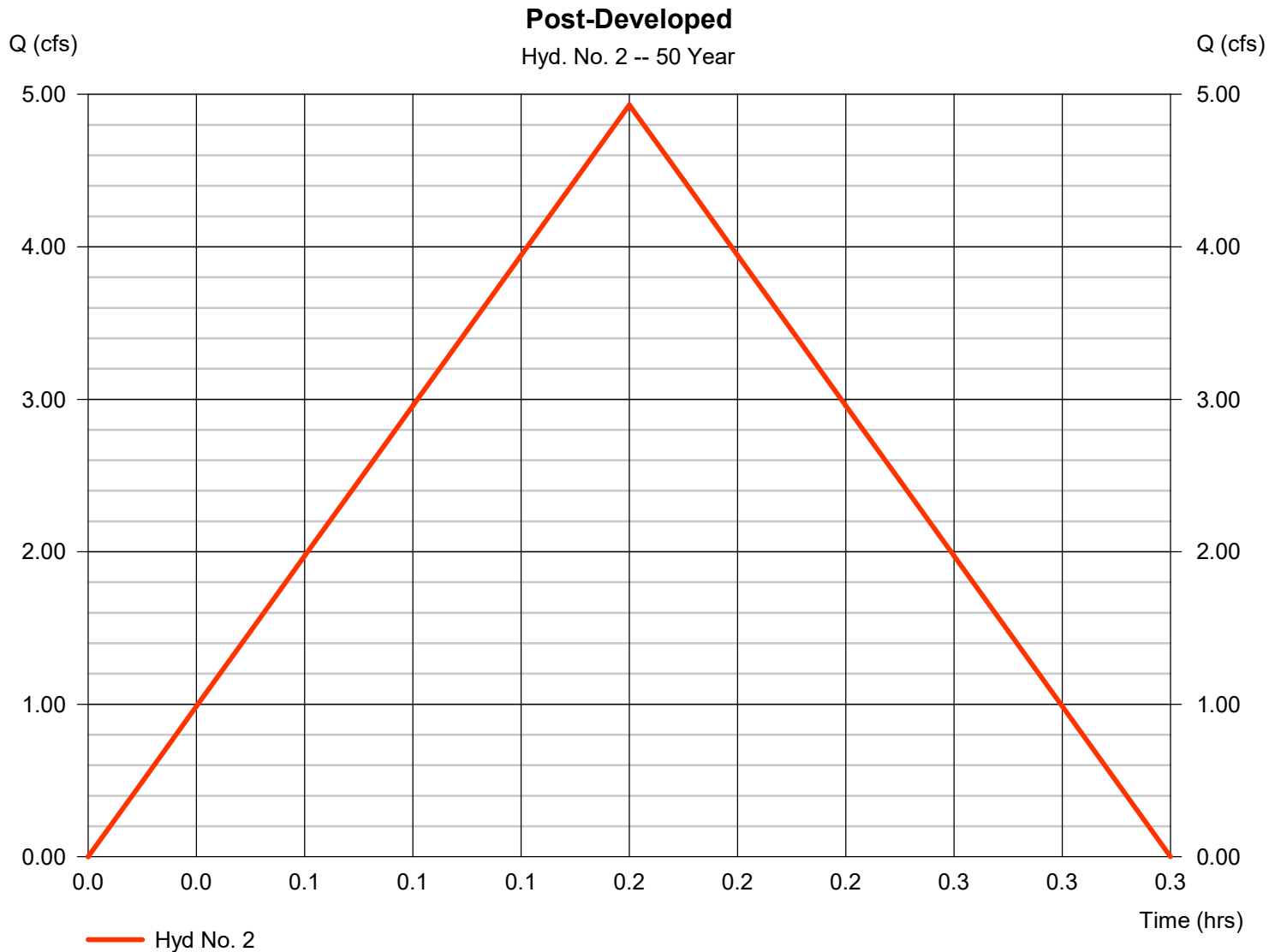


# Hydrograph Report

## Hyd. No. 2

Post-Developed

Hydrograph type	= Rational	Peak discharge	= 4.931 cfs
Storm frequency	= 50 yrs	Time to peak	= 0.17 hrs
Time interval	= 1 min	Hyd. volume	= 2,959 cuft
Drainage area	= 0.901 ac	Runoff coeff.	= 0.81
Intensity	= 6.757 in/hr	Tc by User	= 10.00 min
IDF Curve	= butler county.IDF	Asc/Rec limb fact	= 1/1

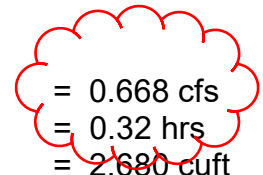


# Hydrograph Report

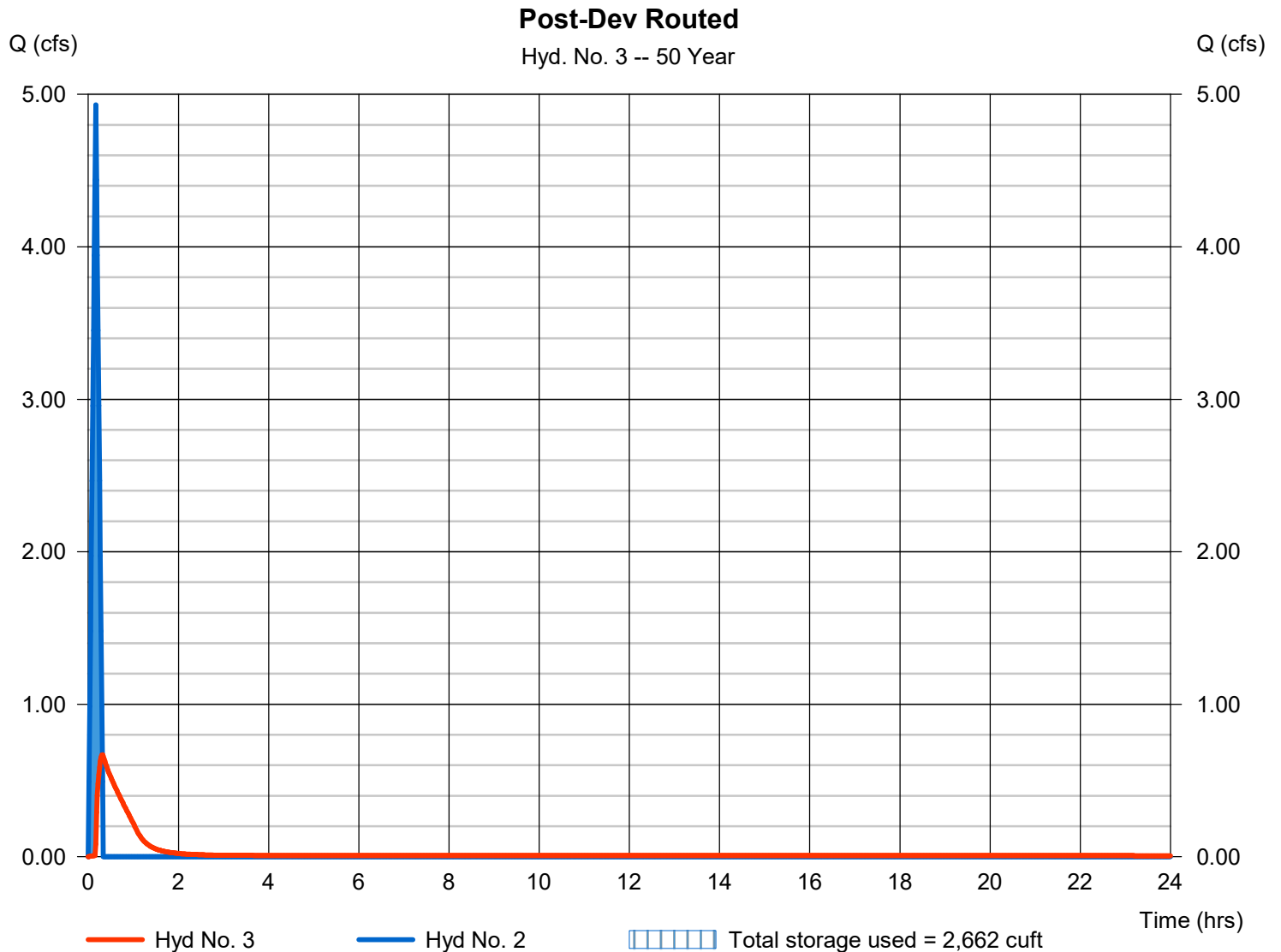
## Hyd. No. 3

Post-Dev Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.668 cfs
Storm frequency	= 50 yrs	Time to peak	= 0.32 hrs
Time interval	= 1 min	Hyd. volume	= 2,680 cuft
Inflow hyd. No.	= 2 - Post-Developed	Max. Elevation	= 564.97 ft
Reservoir name	= UG Chambers	Max. Storage	= 2,662 cuft



Storage Indication method used.



# Hydrograph Report

## Hyd. No. 1

Pre-Developed

Hydrograph type	= Rational	Peak discharge	= 1.568 cfs
Storm frequency	= 100 yrs	Time to peak	= 0.32 hrs
Time interval	= 1 min	Hyd. volume	= 1,787 cuft
Drainage area	= 0.901 ac	Runoff coeff.	= 0.3
Intensity	= 5.800 in/hr	Tc by User	= 19.00 min
IDF Curve	= butler county.IDF	Asc/Rec limb fact	= 1/1



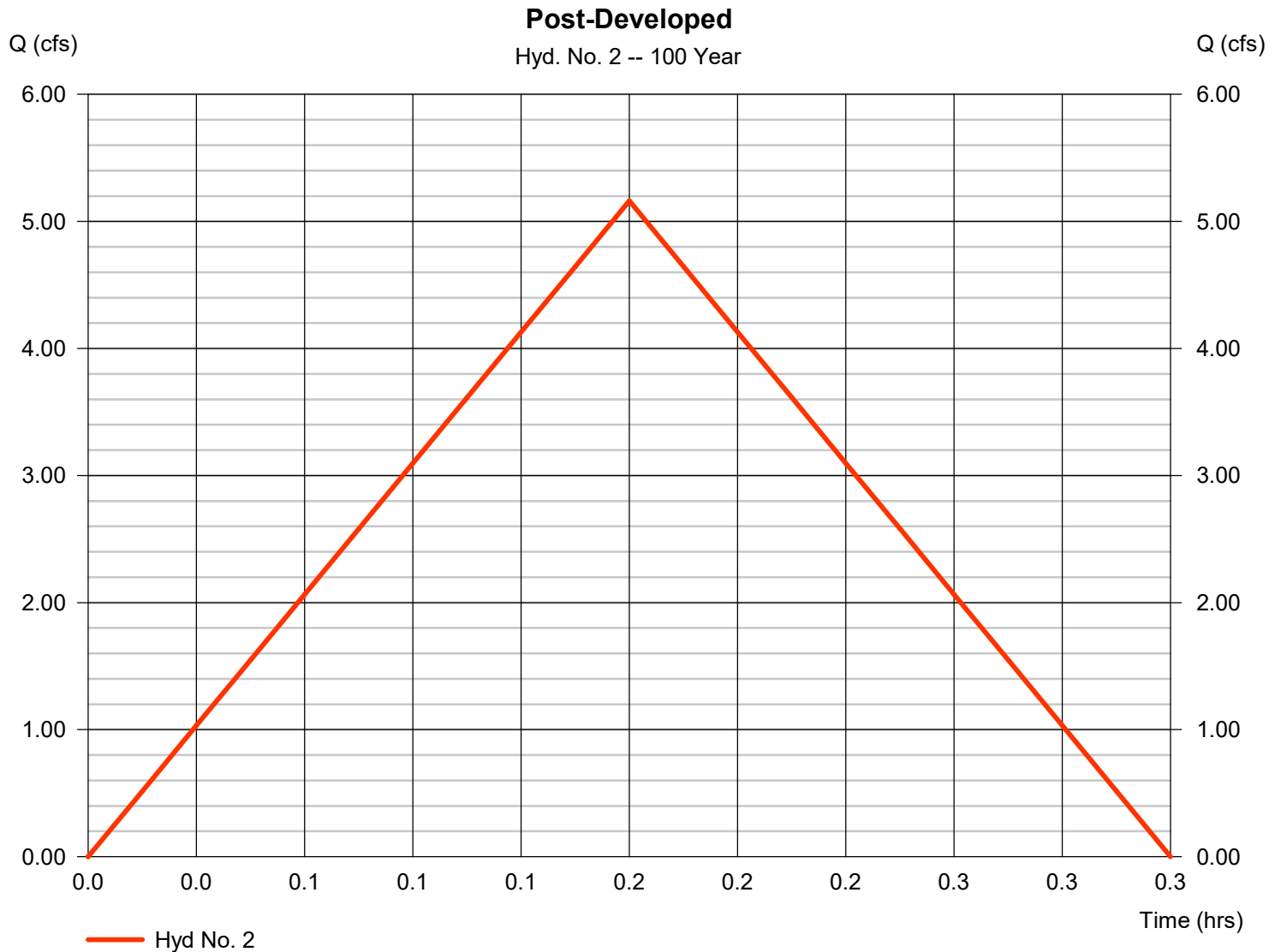


# Hydrograph Report

## Hyd. No. 2

Post-Developed

Hydrograph type	= Rational	Peak discharge	= 5.162 cfs
Storm frequency	= 100 yrs	Time to peak	= 0.17 hrs
Time interval	= 1 min	Hyd. volume	= 3,097 cuft
Drainage area	= 0.901 ac	Runoff coeff.	= 0.81
Intensity	= 7.073 in/hr	Tc by User	= 10.00 min
IDF Curve	= butler county.IDF	Asc/Rec limb fact	= 1/1



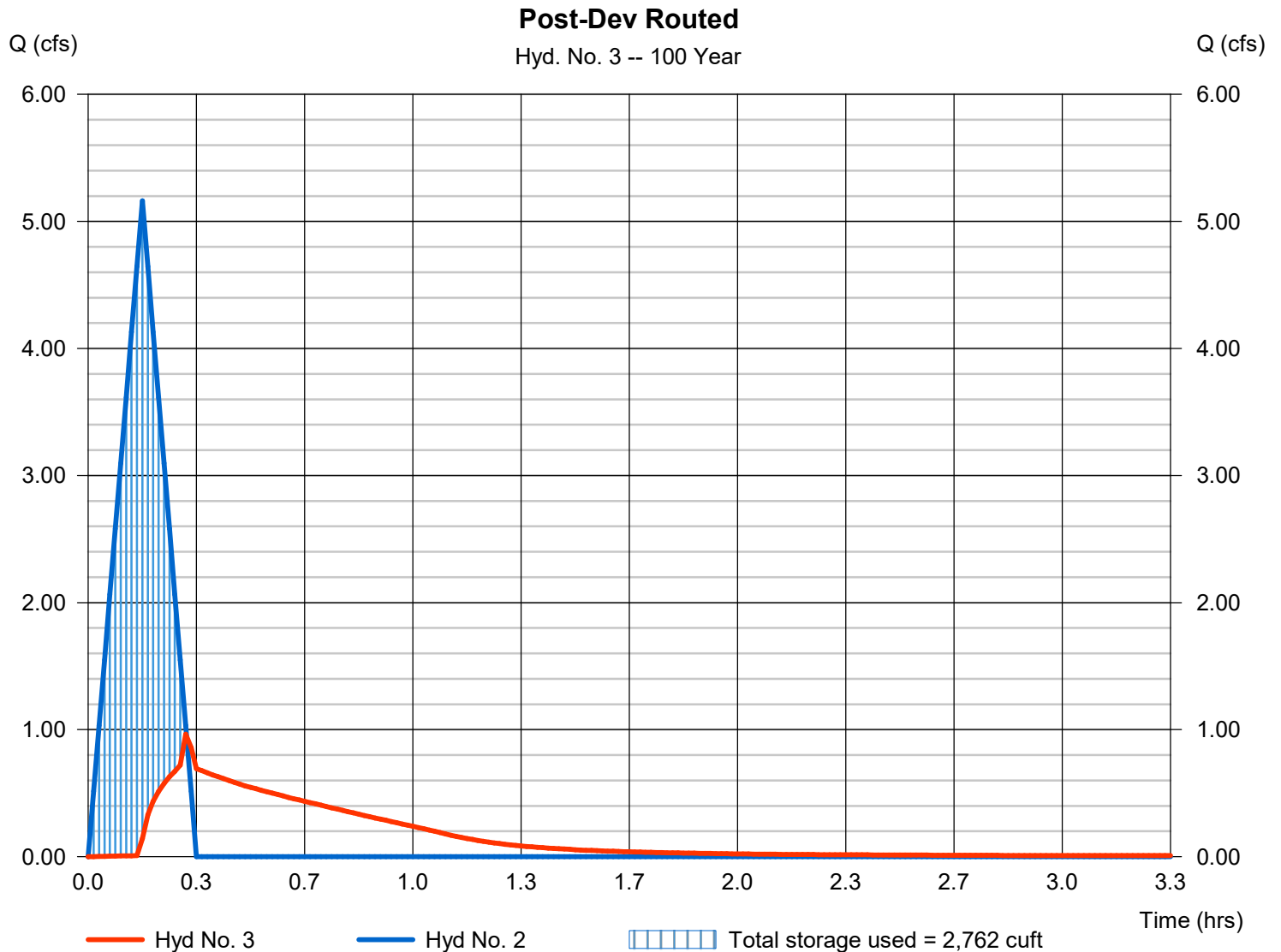
# Hydrograph Report

## Hyd. No. 3

Post-Dev Routed

Hydrograph type	= Reservoir	Peak discharge	= 0.965 cfs
Storm frequency	= 100 yrs	Time to peak	= 0.30 hrs
Time interval	= 1 min	Hyd. volume	= 2,818 cuft
Inflow hyd. No.	= 2 - Post-Developed	Max. Elevation	= 565.16 ft
Reservoir name	= UG Chambers	Max. Storage	= 2,762 cuft

Storage Indication method used.



# Hydraflow Rainfall Report

Return Period (Yrs)	Intensity-Duration-Frequency Equation Coefficients (FHA)			
	B	D	E	(N/A)
1	80.0000	14.0000	1.0000	-----
2	106.0000	17.0000	1.0000	-----
3	0.0000	0.0000	1.0000	-----
5	131.0000	19.0000	1.0000	-----
10	170.0000	23.0000	1.0000	-----
25	230.0000	30.0000	1.0000	-----
50	250.0000	27.0000	1.0000	-----
100	290.0000	31.0000	1.0000	-----

File name: butler county.IDF

**Intensity = B / (Tc + D)^E**

Return Period (Yrs)	Intensity Values (in/hr)											
	5 min	10	15	20	25	30	35	40	45	50	55	60
1	4.21	3.33	2.76	2.35	2.05	1.82	1.63	1.48	1.36	1.25	1.16	1.08
2	4.82	3.93	3.31	2.86	2.52	2.26	2.04	1.86	1.71	1.58	1.47	1.38
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	5.46	4.52	3.85	3.36	2.98	2.67	2.43	2.22	2.05	1.90	1.77	1.66
10	6.07	5.15	4.47	3.95	3.54	3.21	2.93	2.70	2.50	2.33	2.18	2.05
25	6.57	5.75	5.11	4.60	4.18	3.83	3.54	3.29	3.07	2.88	2.71	2.56
50	7.81	6.76	5.95	5.32	4.81	4.39	4.03	3.73	3.47	3.25	3.05	2.87
100	8.06	7.07	6.30	5.69	5.18	4.75	4.39	4.08	3.82	3.58	3.37	3.19

Tc = time in minutes. Values may exceed 60.

Precip. file name: S:\Land Projects 2008\Warren County GIS\Hydro\Warren County-total rainfall.pcp

Storm Distribution	Rainfall Precipitation Table (in)							
	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	2.40	2.80	0.00	3.30	4.25	5.77	6.80	7.95
SCS 6-Hr	0.00	1.80	0.00	0.00	2.60	0.00	0.00	4.00
Huff-1st	0.00	1.55	0.00	2.75	4.00	5.38	6.50	8.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	1.75	0.00	2.80	3.90	5.25	6.00	7.10