



6900 Tylersville Road, Suite A
Mason, Ohio 45040
P 513.336.6600
F 513.336.9365
www.bayerbecker.com

August 24, 2007
Revised October 19, 2007

Mr. Jim Fox
Butler County Department of Development
130 High Street
Hamilton, Ohio 45011

Re: Windisch Road Site, Buildings 6 and 7
Floodway Study

Dear Mr. Fox:



The following items are being submitted in support of a flood hazard area permit for proposed buildings 6 and 7 located in the existing commercial development east of Windisch Road.

1. One copy of the Effective Flood Insurance Study for the Mill Creek.
2. One copy of the Duplicate Effective Model of the Flood Insurance Study.
3. One copy of the Revised Model - Existing Conditions.
4. One copy of the Revised Model - Proposed Conditions.
5. One copy each of the existing and proposed site plans showing the regulatory floodway limits and the study cross section locations.
6. One copy of the floodplain cut/fill balance calculations.

The purpose of this study was to determine the effect of proposed improvements on the 100-year water surface elevation within the regulatory floodway and analyze the flood storage capacity of the floodplain. The study will compare the difference in the water surface elevations between the Revised Model Existing Conditions and the Revised Model Proposed Conditions.

Effective Flood Insurance Study

A copy of the Effective Flood Insurance Study (FIS) EMW-93-C-4106 for the Mill Creek is attached. The study calculated the 10, 50, 100 and 500-year water surface profiles for the natural stream section and determined the limits of the regulatory floodway for a 100-year recurrence interval storm. A summary of the Effective FIS Model results is provided on page 2.

Duplicate Effective Model

A data file was developed using the information contained in the Effective FIS to create a Duplicate Effective Model. Using the data file, the HEC2 computer program calculated the water surface profile within the limits of the regulatory floodway for a 100-year recurrence interval. The Duplicate Effective Model was completed to assure that the Effective FIS model data was input properly prior to further modeling. The study area of the Effective FIS extends for miles north of the limits of the project site, therefore the study area of the Duplicate Effective Model was shortened considerably beginning at Butler County Section E (section number 17.4).

The results of the Duplicate Effective Model matched the published results of the Effective FIS. A summary of the Duplicate Effective Model results is provided on page 2.

Revised Model Existing Conditions

A revised model using current site topography was developed to determine the limits of the regulatory floodway for a 100-year recurrence interval on the existing site. The Duplicate Effective Model was used as the basis for developing the Revised Existing Conditions Model. One cross section, B2, was added to the model to more effectively analyze the existing (and proposed enlarged) retention basin. Section B2 is located between the existing Section B and Section C. Section B2 data was obtained from a combination of aerial and field surveys. In addition to adding Section B2, portions of both Section C and Section D of the Duplicate Effective Model were revised/updated with data from the field survey.

A lake currently resides within the left over bank between the Windisch Road downstream bridge section (section number 17.2) and Butler County Section C (section number 16.0). The lake acts as an ineffective flow area until it overtops the embankment at the south end of the lake at elevation 582.50. The ineffective flow areas of the Windisch Road downstream bridge section and the Section D geometry were blocked out using the X3 card. The right over bank and main channel portions of the sections for this model are identical to those of the Duplicate Effective Model.

All of the sections were artificially terminated on both the left and right sides at stations corresponding to the limits of the regulatory floodway using the ET card. These encroachment stations were computed in the Effective FIS and verified by the Duplicate Effective Model. The encroachment stations for the new Section B2 were obtained from the current mapped floodway limits. A summary of the computed water surface elevations for the Revised Model Existing Conditions is shown below.

Revised Model Proposed Conditions

The Revised Model Existing Conditions was changed to reflect all proposed improvements within the regulatory floodway as shown on the proposed site plan. Changes to the model included the left over bank of sections B2, C and D. Areas of the section below the embankment elevation south of the lake (582.50) were considered ineffective for conveying floodwater and blocked out using the X3 card. The right over bank and main channel portions of the sections for this model are identical to those of the Revised Model Existing Conditions,

A summary of the computed water surface elevations for each model is shown below. The elevations computed using the proposed conditions were found to be equal to or less than those computed using existing conditions.

HYDRAULIC MODELING SUMMARY					
100 Year Water Surface Elevations with Floodway					
Section	Section Number	Effective FIS	Duplicate FIS	Revised Model Existing Condition	Revised Model Proposed Condition
Crescentville Rd. Bridge	14.2	585.46	585.46	585.46	585.46
Crescentville Rd. Bridge	14.3	585.82	585.82	585.82	585.82
Butler County Section A	14.4	586.33	586.33	586.33	586.33
Butler County Section B	15.0	586.59	586.59	586.59	586.59
New Section, B2	15.5	NA	NA	586.88	586.88
Butler County Section C	16.0	587.08	587.08	586.99	586.97
Butler County Section D	17.1	587.15	587.15	587.02	586.98
Windisch Rd. Bridge	17.2	587.23	587.23	587.03	586.99
Windisch Rd. Bridge	17.3	587.24	587.24	587.05	587.02
Butler County Section E	17.4	587.39	587.39	587.23	587.20

Cut/Fill Analysis

The proposed site plan was analyzed from a flood plain volume perspective to ensure any fill placed in the floodplain has been compensated for by an equal or larger volume of cut. Any fill placed above the 100-year water surface elevation was not considered. The 100-year water surface elevation used for the cut/fill analysis is 588.40, which was taken from Station 1590 in the East Fork Mill Creek Flood Control Study prepared by FMSM Engineers in April 2005.

55,426 cubic yards of material are proposed to be placed in the floodplain. 71,298 cubic yards of material are proposed to be cut from the flood plain. The excess cut of 15,872 cubic yards will be utilized for storm water quality volume and storm water detention volume. A copy of the water quality volume calculations and detention calculations are attached. A summary of the cut/fill analysis is shown below. Detailed cut/fill calculations are attached.

FLOOD PLAIN CUT/FILL SUMMARY

Flood Plain Fill = 71,711 C.Y. (Total Fill in Flood Plain)
- 16,285 C.Y. - (Fill Above Flood Plain Elevation, 588.40)
Flood Plain Fill = **55,426 C.Y.**

Total Flood Plain Storage Required = 55,426 C.Y. (Flood Plain Fill)
+ 10,648 C.Y. + (Detention Volume Required)
+ 3,346 C.Y. + (Water Quality Volume Required)
Flood Plain Storage Required = **69,420 C.Y.**

Compensation Volume Provided = **77,503 C.Y.** (Cut in Flood Plain) **INCLUDED EAST BASIN (DO NOT INCLUDE)**
71,298 CY

CONCLUSIONS

This study compared the difference in the water surface elevations between the Revised Model Existing Conditions and the Revised Model Proposed Conditions. The hydraulic modeling showed no rise in the 100-year water surface elevations with floodway. The 100-year water surface elevations slightly decreased in the area of the proposed site due to the excavation of the proposed retention basin.

The floodplain cut/fill analysis showed the proposed site meets requirements for placing fill within the floodplain. As required, the amount of proposed cut (compensation volume) exceeds the amount of proposed fill in the floodplain below the 100-year flood elevation.

Please call with any questions or comments.

Sincerely,

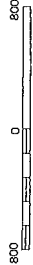


Eric Morris, P.E.

Cc: Al Neyer, Inc.



APPROXIMATE SCALE



NATIONAL FLOOD INSURANCE PROGRAM

FIRM

FLOOD INSURANCE RATE MAP

**BUTLER COUNTY,
OHIO
(UNINCORPORATED AREAS)**

PANEL 50 OF 155

(SEE MAP INDEX FOR PANELS NOT PRINTED)

COMMUNITY-PANEL NUMBER

390037 0050 C

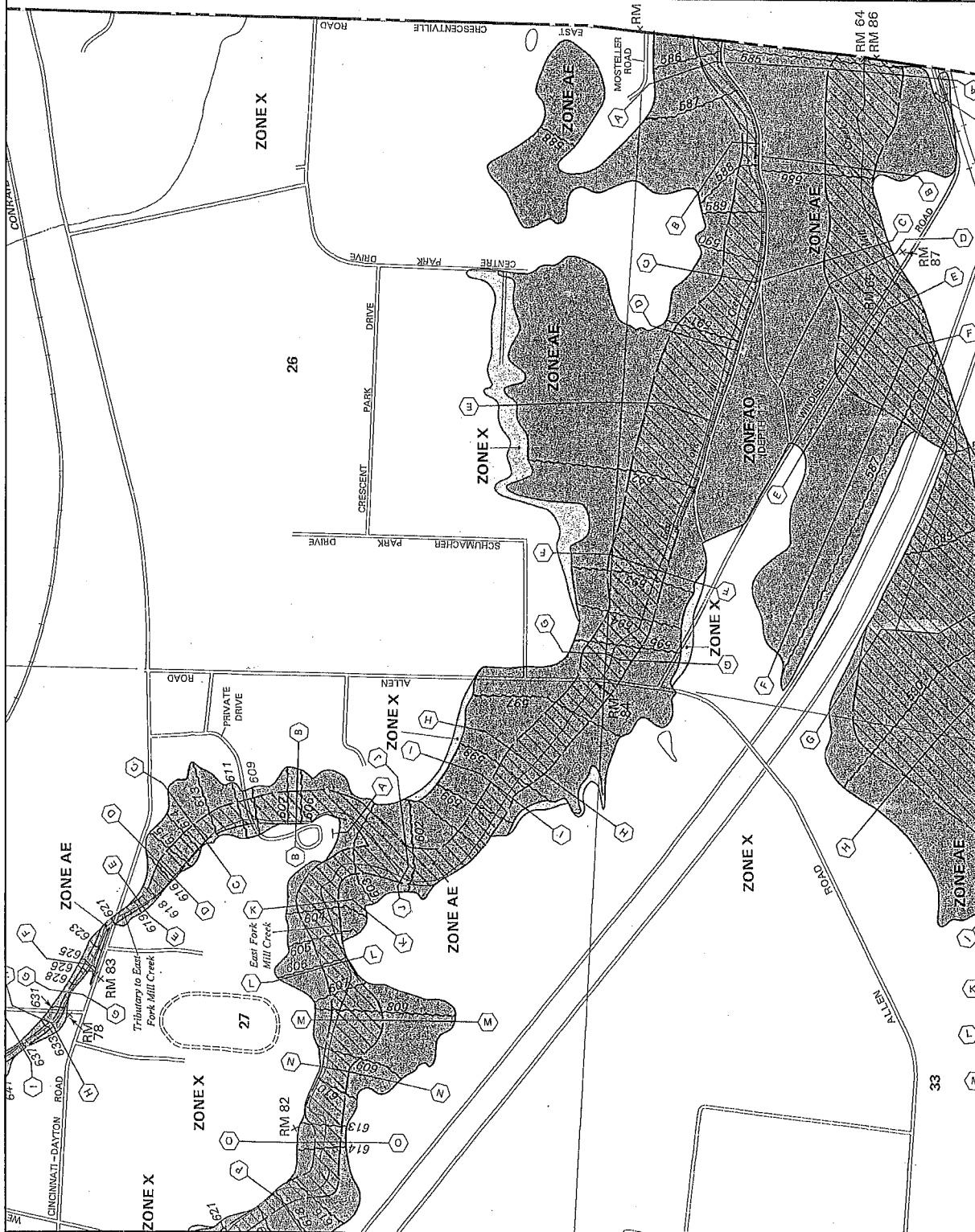
MAP REVISED:

JANUARY 21, 1998



Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using FIRM On-Line. This map does not reflect changes to the flood map since the last time it was printed. For the latest product information about the National Flood Insurance Program flood maps, check the FEMA Flood Map Store at www.fema.gov



1 *****
* HEC-2 WATER SURFACE PROFILES
*
* Version 4.6.2; May 1991
*
* RUN DATE 07APR98 TIME 12:59:39

* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET, SUITE D
* DAVIS, CALIFORNIA 95616-4687
* (916) 756-1104

X	X	XXXXXXXX	XXXXX	XXXXX
X	X	X	X	X
X	X	X	X	X
XXXXXXXX	XXXX	X	XXXXX	
X	X	X	X	X
X	X	X	X	X
X	X	XXXXXXXX	XXXXX	XXXXXXX

1 07APR98 12:59:39

PAGE 1

HEC-2 WATER SURFACE PROFILES
Version 4.6.2; May 1991

THIS RUN EXECUTED 07APR98 12:59:39

T1 Flood Insurance Study-Butler County, Ohio (EMW-93-4106)

T2 100-Year Flood
T3 MILL CREEK

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
	0		4						584.46	
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE
	1	0	-1							

J3 VARIABLE CODES FOR SUMMARY PRINTOUT

115	200	38	66	1	52	4	53	21	22
54									

J5 LPRNT NUMSEC *****REQUESTED SECTION NUMBERS*****

-10 -10
 Starting WSL from MCSHARON.HC2 output
 Peak flow from MILL95.HC1 SR747D
 Current effective floodway will be maintained

QT	5	2467	4009	4465	6335	4465	9.1	1000	1073	1000	1073
NC	0.08	0.08	0.04	0.3	0.5						
ET											1073

Section modified by Crescent Park
 Section revised based on 9/93 Signco drainage plan by Neyer
 and 2/26/94 bridge survey

X1	14.2	17	1000	1073	100	100.0	100.0	585.5	586.4	583	970
X3	10										
GR	586	350	585	795	585	900	584	950			
GR	581.8	1000	575.4	1022	572	1027	569.7	1030	569.7		1030.1
GR	570	1031.4	570	1031.5	572	1050	577.5	1073	582.9		1105
GR	582	1680	586	1740							

ET CRESCENTVILLE ROAD BRIDGE

Top of Road 585.6, low chord 584.5
 Bridge area west of west pier ineffective flow

SB	1.00	1.56	2.80	0.00	19.0	2.00	657	1.91	570
1									

PAGE 2

X1	14.3				32.0	32.0	32.0	585.6	586.4	1000	590	2067	0.13
X2													
X3	10												
BT	-6	350	586		795	585.6	588	1540	585.6				
BT		1073	586.4		1340								
NH	7	1.0	1710	0.06	1988	0.13	2000	0.04					
NH	2100	0.08	2200	1.0	2630								

Butler County Section A
 Section 14.4 2/26/94 survey section u/s of Crescentville Rd
 ET field 9 and 10 original FIS floodway, field 7, 8 optimized floodway
 ET

2400

X1	14.4	18	2000	2067	120	120	1150	583.2	1200
GR	588	750	585.8	925	585.1	581.8	2001	570.9	2002
GR	581.4	1710	579.2	1988	580.8	572	2067	582	2100
GR	570.2	2030	573.2	2032	577.6	579.7			
GR	582	2200	582	2590	589				

NC	0.08	0.12	0.04	0.1	0.3				
NH	7	1.0	713	0.06	984	1000	0.04	1050	0.13
NH	1076	0.12	1131	1.0	1757				

Butler County Section B

Section 15.0 4/94 survey section CM-1

ET					9.1	770	1130	770	1130
X1	15.0	40	1000	1050	800	840			
GR	588.80	192	586.99	196	584.53	584.05	240	583.89	331
GR	583.14	437	582.85	505	581.98	582.00	814	582.66	930
GR	581.96	971	581.11	976	581.99	582.22	994	580.75	1000
GR	573.51	1002	571.58	1003	570.18	570.6	1011	571.97	1017
GR	572.68	1022	573.55	1025	575.49	579.35	1034	579.03	1040
GR	579.26	1045	580.52	1050	582.5	581.39	1067	582.06	1070
GR	583.19	1076	583.19	1082	582.52	583.21	1098	584.53	1131
GR	586.15	1271	585.83	1364	586.09	587.62	1611	588.48	1757

NH	5	1.0	1090	0.06	1530	1543	0.04	1610	0.13
NH	1770								

Butler County Section C, old section D

Original HEC-2 data with both floodplains modified using 1980 topo
Channel area replaced by Windisch Road bridge plan sheet 3/84

Sheet 17/34 "Channel Sections" station 2+50

ET					9.1	1200	1610	1090	1610
X1	16.0	17	1543	1610	630	650			
GR	587.5	500	585	540	584	584.5	1000	584.3	1090
GR	584.0	1190	583	1380	582	580	1543	577	1550
GR	573.2	1559	573.2	1572	583	585	1610	590	1660
GR	595	1678	595.3	1770					

NH	5	1.0	1625	0.06	2730	2748	0.04	2822	0.13
NH	3001								

Butler County Section D, old section E

Section E is modified to exclude East Fork Mill Creek
Section extended 1000' to the north (left bank)
Channel area replaced by Windisch Road bridge plan sheet 3/84

Sheet 17/34 "Channel Sections" station 3+50

ET					9.1	2400	2822	2260	2822
----	--	--	--	--	-----	------	------	------	------

X1	17.1	16	2748	2822	150	100	100	1070	586	1476
GR	0592	0	590	230	588	550	588	2748	573.7	2764
GR	584	1625	584	2400	582	2730	582	2850	595.6	3000
GR	573.7	2797	586	2822	590	2832	594			
GR	595.8	3001								

NC 0.10 0.12 0.055 0.3 0.5
 Section taken off Windisch Road bridge plan 3/84
 Sheet 18/34, "Channel Sections, Mill Creek" Station 5+00

ET										
X1	17.2	20	3000	3118.6	300	10.4	2400	2822	2160	3100
X3	10					130	140	0.843	0.	
GR	592.0	330	590.0	760	589	1000	587.8	1330	592.0	1885
GR	584	2227	583.8	2327	583.6	2427	583.4	2527	583.2	2627
GR	583.0	2727	582.5	2877	581.5	2927	581.5	2977	582	3000
GR	573.6	3026.9	573.6	3082.7	590	3118.6	593.5	3168.6	594.8	3218.6

Windisch Road (BUT-CR 127-0.40)
 Bridge data obtained from Butler County Bridge Plan Project BRZ-0905(1), 1984.
 Top of Road 592, low chord 589
 1959 highwater mark from COE report 585.5 approximately

ET										
SB	1.25	1.5	2.0	0	45	10.4	2600	2680	2160	3100
NC				0.3	0.5	2	976	2	573.6	573.6
X1	17.3	20	3000	3118.6	34	34		0.843		
X2			1	587.8	584.5				0.843	
X3	10									
BT	-20	330	592		760	590		584.5	592	
BT		1330	588		1885	586		1000	589	
BT		2327	585.1		2427	585		2227	585.2	
BT		2627	584.8		2727	584.5		2527	585	
BT		2927	586.1		2977	587.2		2877	585.2	
BT		3026.9	588.7		3082.7	590.4		3000	587.9	
BT		3168.6	593.5		3218.6	594.8		3118.6	592.0	
GR	592.0	330	590.0	760	589			1330	585.5	1885
GR	584	2227	583.8	2327	583.6	1000	587.8	2527	583.2	2627
GR	583.0	2727	582.5	2877	581.5	2927	583.4	2977	582	3000
GR	573.6	3026.9	573.6	3082.7	590	3118.6	581.5	3168.6	594.8	3218.6

NH 4 1.0 2510 0.06 3013 0.04 3059 0.13 3126
 left bank elevation was changed from 588 to 583
 Cross Section revised using Butler County Bridge plans for Windisch Road,
 Channel Station 7+00, and Butler County 1980 Topos (1-26-94).

Section E

ET										
X1	17.4	12	3000	3094	150	9.1	2700	3094	2050	3090
GR	592	430	590	590	588	210	183	1993	584	2510

GR	583	2864	581.8	2910	580.0	3000	573.8	3013	573.8	3059
GR	591.5	3094	600	3126						

1

07APR98 12:59:39

PAGE 4

NC 0.13 0.130 0.05 0.5 0.8
 Change the bank elevations and left overbank elevations based on
 Butler County topo, extend left bank 1000' to the north.

Butler County Section F
 Change left bank from 1500 to 3295 (1-26-94).
 Change left overbank elevations based on Butler County 1980 Topo.

ET						9.1	3020	3358	2560	3358
X1	18.1	16	3295	3358	290	380	390			
GR	590	415	586	435	584	580	584	1010	586	1625
GR	586	2125	584	2690	583.5	3130	582.5	3220	580	3295
GR	575.4	3308	575.4	3328	580	3340	585	3358	590	3469
GR	595	3500								

NC 0.13 0.130 0.05 0.5 0.8
 Change the bank elevations and left overbank elevations based on
 Butler County topo, extend left bank 1000' to the north.

ET						10.4	1833	2377	2660	3264
X1	18.2	19	2900	3264	950	200	280	0.67		
X3	10							592.3	607.1	
GR	590	300	584	400	584	1000	585	2360	585	2490
GR	584.9	2670	584.5	2800	584	2900	584	3000	582.8	3094
GR	576.4	3106	576.1	3119	576.4	3132	584.1	3140	584.8	3154
GR	590	3264	610	3325	611	3364	613.5	3464		

Interstate 75
 Top of Road 612.9, Low steel 601.8
 1959 highwater mark from COE report 586.5 approximately
 Bridge model use original FIS data

ET						10.41	2042	2246	2660	3264
SB	0.9	1.5	2.5	0	28	6	3207	4.0		
X1	18.3	0	0	0	390	410	410			
X2	0	0	1	601.8	592.3				0.67	
X3	10							592.3	613.1	
BT	-12	300	593.5	590	400	593	584	1000	592.3	584
BT		2360	596.5	585	2490	597	585	2670	600	584.9
BT		2800	603	584.5	2900	605.3	584	3000	607.8	601.8
BT		3264	613.1	607.1	3364	614.2	611	3464	615.7	613.5

Butler County old section G
 bank location changed

GR	578	2033.1	578	2033.9	578	2034	578	2067	578	2067.1
GR	578	2067.9	578	2068	593.3	2102	589	2103	585	2157
GR	584.8	2477	584.8	2700	594	3200				

Section 20.3 Mulhauser Road extension
 As of 11/94, the Mulhauser Road Bridge was not in place, but the fills were completed. Two tests were run, one with the fill but on bridge, one with bridge and fills. The WSLs are identical. Therefore, although, the bridge was not in place, the HEC-2 model will include the bridge
 Top of Road 595, LC 593.3

ET					9.1	2000	2102	1300	2640
SB	1.05	1.5	2.5	0	2	910.9	2	578	578
NC			0.03						
X1	20.3				32	32			
X2			1	593.3					
X3	10						595	595	595
BT	-23	620	595	594	595	590	965	595	586
BT		1458	595	582.5	595	582.8	1800	595	583.7

BT		1900	595	583.8	1999	595	585	2000	595	593.3
BT		2033	595	593.3	2033.1	595	578	2033.9	595	578
BT		2034	595	593.3	2067	595	593.3	2067.1	595	578
BT		2067.9	595	578	2068	595	593.3	2102	595	593.3
BT		2103	595	589	2157	595	585	2477	603	584.8
BT		2700	607	584.8	3200	610	594			

NH	7	0.15	1426	0.06	1980	0.13	2000	0.04	2040	0.12
----	---	------	------	------	------	------	------	------	------	------

NH 2074 0.08 2780 0.2 2900
 Section 20.4 from Mauhauser Road existing profile and Butler County Interactive Graphics Study

Section I

ET						9.1	1700	2400	1280	2580
X1	20.4	21	2000	2040	348	348	348			
GR	592	800	590	890	588	940	586	1100	582.8	1426
GR	583.2	1700	584.	1900	585	1980	584.8	2000	578.1	2009
GR	578.5	2031	585.8	2040	589	2074	585	2127	584.4	2280
GR	584.1	2400	585	2526	586	2710	588	2750	590	2780
GR	592	2900								

NC				0.1	0.3		2882	0.06	3295	1.0
NH	5	1.0	2375	0.06	2825	0.045				
NH	3560									

Between Sections J to N, only changes made to original HEC-2 data are change bank stations, remove X3 cards and use NH card to model ineffective

[illegible]

	5	1600	0.06	2205	0.045	2302	0.06	3320	1.0
NH									
NH	3912								

[illegible]

NH	6	0.06	945	0.12	1010	0.040	1075	0.12	1098	0.06
NH	2270	1.0	2815							

Added section, section coded from Union Township 1980 topographic map and Butler County interactive map
Section 21.06 (old section O) deleted, this section will be section O 9.1

9.1

07APR98 12:59:39

PAGE 8

	26.1	35	1000	1080	700	700		
X1	GR	200	592	230	280	588	360	945
	GR	978	594	1000	1010	584	1028	1030
	GR	1050	584	1054	1075	592	1080	1091
	GR	1098	588.7	1200	1260	587.9	1470	1480
	GR	1485	590	1495	1508	586.2	1650	2200
	GR	2270	590	2280	2300	592	2325	2340
	GR	2360	588	2470	2560	592	2740	2815

	NC	NH
6	0.06	0.06
.3	945	1098
.5	1010	1075
0.12	0.040	0.12
0.12	0.040	0.06

[illegible]

GR	594.0	533	592.0	540	590.0	543	586.0	551	586.6	563
GR	590.0	572	596.0	580	596.0	585	594.0	590	592.0	592
GR	591.0	640	592.0	694	592.0	709	592.0	719	594.0	723
GR	596.0	731	597.8	739	597.8	745	596.0	758	594.0	765

1 07APR98 12:59:39 PAGE 10

GR	592.0	775	590.0	781	590.0	805	590.4	841	590.0	891
GR	590.0	1276	595.0	1371	600.0	1411				

NH	5	0.06	980	0.100	1013	0.035	1044	0.100	1098	1.0
NH	1842									

FIS CROSS-SECTION S
Area south of RR not effective
use x3 card for storage calculation purpose

ET										
X1	29.1	39	996	1083	1295	9.1	1300	0	440	1080
X3	10							594	599	0
GR	600.0	100	598.0	113	596.0	138	594.0	185	594.0	290
GR	594.0	387	593.6	476	594.0	639	594.0	692	594.0	715
GR	596.0	980	598.0	989	598.0	996	596.0	1001	594.0	1004
GR	592.0	1009	590.0	1013	586.8	1026	586.8	1033	590.0	1044
GR	592.0	1051	594.0	1060	596.0	1065	598.0	1073	599.0	1083
GR	599.0	1089	598.0	1098	596.0	1101	594.0	1105	592.0	1114
GR	590.3	1127	592.0	1142	592.0	1165	592.0	1230	593.2	1522
GR	594.0	1769	596.0	1820	598.0	1830	600.0	1842		

NC 0.06 1.0 0.035 0.3 0.5
Butler County Section T replaced by relocated 747 bridge downstream section.
Cross section and bridge data taken from Butler County bridge plans for
State Route 747 (BUT-747-2.12, Feb. 1993). (sheets 14-18/50)

ET										
X1	29.2	23	2000	2084.5	1300	9.1	1115	0.872	1480	2085
X3	10					950		598.8	600.20	
GR	604	185	599.9	966	599.0	1216	597.6	1266	596.9	1366
GR	595.9	1466	595.6	1666	595.3	1766	596.7	1866	595.8	1966
GR	596.3	2000	588.8	2015	588.8	2069.5	596.3	2084.5	599.5	2119
GR	593.4	2166	594.1	2266	593.9	2366	594.2	2566	595.2	2966
GR	596.5	3166	602.2	3216	605.0	3280				

NC SR 747 built in 1994
TOR 600.2 LS=598.5

ET										
SB	1.05	1.5	2.8		54.5	9.1	654.5	1.5	1480	2085
X1	29.3	23	2000	2084.5	80	2	80	0.872	589.50	589.5

[illegible]

Between Section 22.21 to 23.41, very little change was made to the original data for the Butler County FIS, NH cards were used for uneffective area sections 22.98 and 23.08 were extended on the left bank using USGS topo 1959 highwater from COE report 598 approximately

NH	5	0.06	1700	0.06	2085	0.035	2128	0.10	2150	0.25
NH	3175									
Butler County Section U										
Original section 22.21										
Use X3 card to prevent flow over RR track unless overtopped										
ET						9.1				
X1	29.4	18	2085	2150	200	230	230		1500	2150
X3	10									
GR	605	370	600	420	600	490	597.5	1250	597.5	1700
GR	597.5	2000	597.4	2070	595	2085	590	2095	589.6	2100
GR	589.6	2112	590	2118	595	2128	601	2150	595	2252
GR	597.5	2282	600	3150	604.5	3175				
NH	4	0.06	1766	0.040	1823	0.1	1842	0.25	2818	
Butler County Section V										
Peak discharges from MILL.HC1 Tyler										
QT	5	1513	2396	2706	3723	2706				
ET						9.1			1200	1842
X1	30.0	19	1766	1842	875	900	900			
X3	10									
GR	601	280	600	500	597.5	1200	597	1416	595	1766
GR	590	1776	589.6	1784	589.6	1792	590	1798	595	1810
GR	600	1823	602	1842	595	1880	595	2300	597.5	2685

GR	600	2720	602.5	2770	605	2810	610	2818
NC				0.1	0.3			
NH	4	0.06	2422	0.042	2488	0.10	2496	0.25
Butler County Section W								
ET								
X1	31.0	20	2422	2488	512	9.1	1800	2488
X3	10					512		
GR	602.5	300	600	500	597.5	2195	595	2422
GR	589.8	2444	589.8	2452	590	2459	595	2467
GR	602	2488	600	2496	595	2530	595	2860
GR	597.5	3410	600	3440	602.5	3510	605	3545
NH	4	0.06	3380	0.045	3450	0.10	3460	0.25
Butler County Section X								
ET								
X1	32.0	26	3380	3450	1550	9.1	2541	3450
X3	10					1550		
GR	610	300	605	680	602.5	1110	600	1500
GR	597.5	2432	598	3352	598	3362	597.5	3372
GR	595	3383	590	3392	589.9	3400	589.9	3412
GR	595	3430	600	3438	603.2	3450	600	3460
GR	595	4185	597.5	4375	600	4400	602.5	4580
GR	610	4900						

NH	4	0.06	3338	0.043	3385	0.12	3392	0.25	4990	
Butler County Section Y										
ET						9.1			3229	3392
X1	33.0	21	3338	3392	1450	1090	1120			
X3	10									
GR	610	0	605	100	603	400	602.5	1920	600	2920
GR	597.5	3221	595	3338	590	3345	590	3365	595	3375
GR	600	3385	605	3392	605.4	3400	605	3402	600	3415
GR	596	3970	600	4355	602.5	4445	605	4680	607.5	4750
GR	610	4990								
NH	4	0.06	3250	0.045	3320	0.12	3332	0.25	4850	
Butler County Section Z										
ET						9.1			3250	3332
X1	34.0	27	3250	3332	450	530	540			
X3	10									
GR	610	0	606	140	603	400	602.5	2060	600	3020
GR	600	3250	595	3262	591.3	3268	591.3	3283	595	3292

GR	600	3320	605	3332	605	3348	600	3363	595	3395
GR	595	3750	597.5	4065	600	4095	601	4210	605	4295
GR	605	4311	600	4320	600	4338	602.5	4370	602.5	4550
GR	605	4730	607.5	4850						

NH 4 0.06 1910 0.045 1943 0.12 1958 3780

Butler County Section AA

ET						9.1	1430		1958	
X1	35.0	20	1910	1958	600	620	575			
X3	10									
GR	605	0	600	1260	600	1895	600	1910	595	1918
GR	592.4	1925	592.4	1938	595	1943	600	1950	605	1958
GR	605	1972	600	1988	595	2021	595	2655	597.5	2718
GR	600	2745	600	3002	605	3030	607.5	3140	610	3780

Butler County Section AB

NC	0.06	0.25	0.045							
ET						9.1	1280		2219	
X1	36.0	22	2182	2219	1112	1120				
X3	10									
GR	610	0	605	100	600	1030	600	2115	600	2182
GR	595	2192	594.6	2201	594.6	2213	595	2218	605	2219
GR	600	2263	597.5	2303	597.5	2700	595	2740	595	2760
GR	597	3060	597.5	3330	600	3410	600	3560	605	3610
GR	607.5	3630	610	3980						

NH	6	1.0	1500	0.060	3848	0.12	3911	0.040	3911	0.12
----	---	-----	------	-------	------	------	------	-------	------	------

NH 3951 0.060 4300

First Cross Section of New Study

Section surveyed 10/15/93

Section AC

ET						9.1	2900		3911	
----	--	--	--	--	--	-----	------	--	------	--

1

07APR98 12:59:39

X1	37.0	25	3895	3911	2600	2600	2600	1500	601.2	2500
GR	610	0	605	500	600	1000	600	3500	600.6	3634
GR	600.9	2780	601.2	3130	600.7	3390	600.5	3891	597.79	3895
GR	600.8	3754	601.6	3848	608	3870	599.24	3919	607.69	3931
GR	597.44	3899	597.06	3907	597.52	3911	603.1	4260	610	4300
GR	602.9	3951	603.88	4009	603	4157	604			

NH 4 1.0 1600 0.06 2000 0.045 2049

Survey section 53'd/s of Seaward Road, moved downstream, lower .79'

Section AD

ET						9.1	1900		2049	
----	--	--	--	--	--	-----	------	--	------	--

X1	38.1	13	2000	2049	900	900	1860	603	1970
GR	610	150	605	600	604	603	2015	598.04	2019
GR	608.94	1985	605.14	2000	597.64	597.44	2250		
GR	602.74	2034	606.34	2049	606.84	610			

NC 0.08 0.08 0.045 0.5 0.8

SEWARD ROAD BRIDGE (Special BRIDGE ROUTINE)
 Bridge and channel section from Site Plan, Seward Road bridge No. 02.55
 dated 6/93, prepared by Lockwood, Jones & Beals
 TOR =610.6, LS=606.7

Berm northeast of Seward elevation 609

ET						10.4			
X1	38.2	10	2000	2047	300	220	250		
X3	10								
GR	620	600	610	700	606	1690	606.7	610	607.7
GR	598	2031	607	2047	607	2090	610	598	2015
								620	2780
ET						10.4			
SB	1.0	1.6	2.6		16	0.1	306	2000	2047
X1	38.3				32	32	2.0		
X2			1	606.7	609				
X3	10								
BT	-10	600	620	620	700	610	609	610	606
BT		2000	610.14	606.7	2015	610.2	1690	607.8	607
BT		2047	610.35	607	2090	610.96	2031	610.4	610
BT		2780	620	620			2290	612.42	
NH	5	1.0	1930	0.06	2000	0.04	0.06	2075	1.0
NH	4900								

Survey section 1.6
 1959 highwater from COE report 608 approximately
 Section AE

ET						9.1		1960	2223
X1	38.6	15	2000	2030	80	80			
GR	620	600	610	700	606.73	1930	1950	603.93	2000
GR	598.23	2003	597.73	2014	598.23	2025	2030	605.83	2042
GR	609.73	2065	606.5	2075	606.5	4023	4600	620	4900

leaving the contraction/expansion coeff

1

07APR98 12:59:39

PAGE 14

NH	5	1.0	700	0.06	994	0.045	1017.5	1400	1.0
NH	3400								

Survey section 1.7 10/14/93
 Section AF

ET	X1	39.0	18	994	1017.5	700	9.1	700	894	1276
GR	615	30		610	80	608	700			
GR	607.83	978		606.2	984	602.91	300	607.8	607.82	890
GR	604.5	1017.5		611.08	1030	606.68	994	599.61	599.61	1010
GR	608	3000		610	3200	620	1042	606.7	608	1400
NH		5	0.06	1966	0.12	2020	0.04	2050	2136	0.06
NH	3300									
ET	X1	40.1	17	2000	2070	310	10.4			
GR	620	1100		610	1200	608.3	310	608.6	608.6	1934
GR	608.5	1944		608.4	1966	606.7	1700	1800	599.5	2035
GR	600.4	2050		606.7	2070	608.4	2000	2020	609	2136
GR	610	2300		620	3300		2104	2126		
NC										
NH	5	0.06		1966	0.3	0.5				
NH	3200				0.12	2019.2	0.040	2050.7	2126	0.10
PRIVATE DRIVE BRIDGE (NORMAL BRIDGE ROUTINE)										
TOR =612.2 LS=609.4										
X1	40.2	26		2019.2	2050.7	100	100	100		
X3	10								610	
GR	620	1140		608.3	1500	609.1	1860	608.5	608.4	1966
GR	606.7	2000		600.7	2019.2	600.4	2021.6	599.6	599.4	2026.5
GR	599.4	2028.9		599.6	2031.3	599.7	2033.7	599.7	599.7	2036.25
GR	599.4	2038.6		599.7	2041.1	600.7	2043.5	601.6	602.6	2048.3
GR	603.45	2050.7		606.7	2070	610	2104	613.5	610	2136
GR	620	3200								
NH	5	0.06		1966	0.12	2019.2	0.040	2050.7	2126	0.10
NH	3200									
X1	40.3									
X3	10						1	1		
BT	-26	1140		620	620	1500	608.3	608.3	610	609.1
BT		1944		610	610	1966	610	1860	609.1	610
BT		2019.2		612.2	605.45	2021.6	612.2	2024.0	612.2	608.98
BT		2026.5		612.2	609.4	2028.9	612.2	2031.3	612.2	607.55
BT		2033.7		612.2	599.7	2035	612.2	2036.25	612.2	599.7
BT		2038.6		612.2	607.55	2041.1	612.2	2043.5	612.2	609.4
BT		2045.9		612.2	608.98	2048.3	612.2	2050.7	612.2	605.45
BT		2070		610	610	2104	610	2126	613.5	613.5
BT		2136		610	610	3200	620	620		

NH	5	0.06	1966	0.12	2019.2	0.040	2050.7	0.12	2126	0.10
NH	3200									
X1	40.4			12			12			
X2							1			
X3	10							608	610	
NH	5	0.060	1966	0.12	2019.2	0.040	2050.7	0.12	2126	0.10
NH	3200									
X1	40.5	26	2019.2	2050.7	1	1	1			
GR	620	1140	608.3	1500	609.1	1860	608.4	1944	608.4	1966
GR	606.7	2000	600.7	2019.2	600.4	2021.6	599.6	2024.08	599.4	2026.5
GR	599.4	2028.9	599.6	2031.3	599.7	2033.7	599.7	2035	599.7	2036.25
GR	599.4	2038.6	599.7	2041.1	600.7	2043.5	601.6	2045.92	602.6	2048.34
GR	603.45	2050.7	606.7	2070	608.4	2104	613.5	2126	610	2136
GR	620	3200								
NC										
NH	5	0.08	1966	0.1	0.3	0.040	2050	0.10	2126	0.10
NH	3100			0.1	2020					
ET						9.1			1925	2050
X1	40.6	13	2020	2050	70	70	70			
GR	620	1000	610	1300	610	1944	608.4	1966	606.7	2000
GR	600.4	2020	599.5	2035	600.4	2050	606.7	2070	608.4	2104
GR	613.5	2126	610	2130	620	3100				
NC	0.09	0.09	0.045							
Survey section 2.7										
ET						10.4				
X1	41.0	12	1000	1075	640		640			
GR	620	20	610	460	609.65	920	608.55	1000	606.25	1027
GR	600.65	1043	600.15	1048	600.55	1054	609.45	1075	609.65	1175
GR	610	1510	620	2020						
Section AG										
Survey section 2.8										
ET						5.4			900	1064
X1	42.0	12	1000	1037	810	740	800		2.0	
GR	620	70	611.25	910	612.25	1000	609.05	1007	601.95	1012
GR	602.25	1015	602.25	1020	607.85	1037	611.95	1084	610	1200
GR	610	1400	620	1960						
NC				0.3	0.5					
Survey section 3.1										
ET						9.1			1000	1105
X1	43.1	9	1000	1070	800	800	800			
GR	620	820	615.9	942	614.7	1000	607.8	1013	607.1	1024

GR 607.2 1030 613.6 1070 616.4 1092 620 1850
 Tylersville Road
 Elevations Taken From Butler County Cut & Fill Plan Dated 6-86
 TOR=622.8, LS=620.9

07APR98 12:59:39

ET	43.2	17	1000	1105	100	10.4	100	620	609.9	1025
X1	10								617	1105
X3									614.9	1550
GR	623.2	595	618.4	906	618.1	950	620	1000		
GR	607	1035	607	1045	608.3	1055		1075		
GR	615.7	1150	614.2	1250	613.1	1350		1450		
GR	617.4	1650	620	1700						

NC			0.035							
ET						10.41				
SB	0.9	1.6	2.6		50	3.0		1.92	607.0	607.0
X1	43.3				50	50				
X2			1	620.9	622.4					
X3	10									
BT	-12	595	623.2		900	623.2		622.8	620	
BT		1000	622.8		1105	622.4		950	623.0	
BT		1250	621.7		1350	621.3		1150	622.1	
BT		1550	620.4		1650	620.1		1450	620.8	
								1700	620.0	

NC 0.06 0.06 0.04
 Section 43.4 and 44.0 from Millers Run subdivision Grading Plan
 Sheet 7A/11 (Bayer-Becker Engineering)

Section AH

ET	43.4	16	1000	1124	200	10.4	200			
X1	620	840	618	910	616	937		1000	612	1040
GR	610	1060	608	1066	607.5	1067		1080	608	1081
GR	610	1104	612	1114	614	1124		1220	616	1300
GR	618	1308								

NC				0.1	0.3					
Section AI										
X1	44.0	13	1000	1050	600	560	700			
GR	620	580	616	772	614	890		1000	610	1014
GR	608	1015	608	1029	610	1030		1040	614	1050
GR	616	1064	618	1170	620	2060				

Section 45.0 from Schul Estates Section Six Plan (Bayer Becker Eng., 1"=50')
 July 13. 1988

ET	45.0	18	1000	1042	450	9.1	900	1142
X1						450		
GR	621	485	618	525	621	577	618	962
GR	618	982	616	990	614	996	610	1003
GR	609.5	1008	609.5	1036	610	1041	614	1043
GR	616	1044	616	1128	620	1500		

Section 46.0 from Schul Estates Section Six plan, Section D

Section AJ

I

07APR98 12:59:39

PAGE 17

ET	46.0	10	1000	1047	520	9.1	836	1047
X1						520		
GR	620	605	620	635	621	680	618	1000
GR	610	1010	610	1030	616	1047	620	1200
NC	0.08	0.08	0.045					

Cross Section 47.0 From Schul Estates Plat Plan (Bayer Becker Eng., 1"=100')

Section AK

三

47.0 11

628 625

618 · 1000

630 1212

Section AL

	48.0	13	1000	1058	1040	1040	1040	
X1								
GR	628	850	626	895	624	982	622	1000
GR	618	1006	617.5	1012	617.5	1040	618	1058
GR	622	1068	624	1150	626	1266		
Cross Section From Schul Estates Grading Plan (Bayer Becker Eng., 1"=50')								
Section AM								

49.0 14

630 976

621.8 1024

624 1065

07APR98 12:59:39

PAGE 18

T1	Flood Insurance Study	EMW-93-C-4106
T2	Floodway Analysis	
T3	Mill Creek	Butler County, Ohio

T1 Flood Insurance Study EMW-93-C-4106

T2 Floodway Analysis

T3 Mill Creek

Butler County, Ohio

J1 ICHECK INQ NINV IDIR STRT METRIC HVINS Q WSEL FQ
 0 6 585.46
 J2 NPROF IPLOT PRFVS XSECV XSECH FN ALLDC IBW CHNIM ITRACE

2 -1

1 07APR98 12:59:39

PAGE 19

THIS RUN EXECUTED 07APR98 12:59:40

 HEC-2 WATER SURFACE PROFILES
 Version 4.6.2; May 1991

NOTE- ASTERISK (*) AT LEFT OF CROSS-SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST

MILL CREEK

SUMMARY PRINTOUT

SECNO	CUMDS	CWSEL	DIFKWS	TOPWID	SSTA	STCHL	STCHR	ENDST
14.200	.00	584.46	.00	73.00	1000.00	1000.00	1073.00	1073.00
14.200	.00	585.46	1.00	73.00	1000.00	1000.00	1073.00	1073.00
14.300	32.00	585.04	.00	73.00	1000.00	1000.00	1073.00	1073.00
14.300	32.00	585.82	.78	103.00	970.00	1000.00	1073.00	1073.00
*	14.400	152.00	585.73	.00	1673.60	937.71	2000.00	2611.31
*	14.400	152.00	586.33	.61	600.00	1800.00	2067.00	2400.00
*	15.000	992.00	585.98	.00	1160.12	198.86	1000.00	1421.81
*	15.000	992.00	586.59	.61	360.00	770.00	1050.00	1130.00
16.000	1642.00	586.50	.00	1109.08	515.95	1543.00	1610.00	1625.03
16.000	1642.00	587.08	.58	520.00	1090.00	1543.00	1610.00	1610.00

* 17.100	1742.00	586.64	.00	1478.20	1345.41	2748.00	2822.00	2823.61
17.100	1742.00	587.15	.51	562.00	2260.00	2748.00	2822.00	2822.00
17.200	1882.00	586.70	.00	1278.96	1395.75	2580.81	2680.79	2674.71
17.200	1882.00	587.23	.53	455.49	2220.19	2580.81	2680.79	2675.68
17.300	1916.00	586.71	.00	1280.85	1393.88	2580.81	2680.79	2674.73
17.300	1916.00	587.24	.53	456.88	2218.83	2580.81	2680.79	2675.70
17.400	2099.00	586.82	.00	1306.29	1778.46	3000.00	3094.00	3084.75
17.400	2099.00	587.39	.57	1035.88	2050.00	3000.00	3094.00	3085.88
18.100	2489.00	586.95	.00	2971.05	430.25	3295.00	3358.00	3401.30
18.100	2489.00	587.48	.53	798.00	2560.00	3295.00	3358.00	3358.00
18.200	2769.00	587.40	.00	207.06	2042.00	2042.00	2285.88	2249.06
18.200	2769.00	587.84	.43	213.22	2042.00	2042.00	2285.88	2255.22
1 07APR98 12:59:39								
SECNO	CUMDS	CWSEL	DIFKWS	TOPWID	SSTA	STCHL	STCHR	ENDST
18.300	3179.00	587.44	.00	207.67	2042.00	2042.00	2285.88	2249.67
18.300	3179.00	587.87	.42	213.65	2042.00	2042.00	2285.88	2255.65
18.400	3409.00	588.06	.00	239.78	3161.61	3190.00	3240.00	3401.39
18.400	3409.00	588.36	.30	233.00	3162.00	3190.00	3240.00	3395.00
19.000	4931.00	589.88	.00	2703.19	955.14	2290.00	2360.00	3835.36
19.000	4931.00	590.06	.18	1500.00	1900.00	2290.00	2360.00	3400.00
20.100	5801.00	590.12	.00	2494.60	657.60	1799.00	1855.00	3152.20
20.100	5801.00	590.34	.21	1570.00	1000.00	1799.00	1855.00	2570.00
20.200	6851.00	590.12	.00	88.09	2006.85	2000.00	2102.00	2094.94
20.200	6851.00	590.40	.28	89.31	2006.25	2000.00	2102.00	2095.56
20.300	6883.00	590.16	.00	88.25	2006.77	2000.00	2102.00	2095.02
20.300	6883.00	590.44	.27	89.46	2006.18	2000.00	2102.00	2095.63
20.400	7231.00	590.90	.00	1984.88	849.34	2000.00	2040.00	2834.22
20.400	7231.00	591.14	.23	1300.00	1280.00	2000.00	2040.00	2580.00
21.000	8501.00	590.97	.00	2426.94	1072.58	2825.00	2882.00	3499.52

*	21.000	8501.00	591.21	.24	1520.00	1620.00	2825.00	2882.00	3140.00
*	22.000	8991.00	591.03	.00	1726.02	723.77	2673.00	2748.00	3317.73
*	22.000	8991.00	591.26	.23	803.82	1200.00	2673.00	2748.00	2860.00
*	23.000	9391.00	591.27	.00	1809.50	491.28	2632.00	2715.00	3394.05
*	23.000	9391.00	591.51	.24	661.72	1160.00	2632.00	2715.00	2900.00
*	24.000	10131.00	591.86	.00	3488.36	749.15	2555.00	2660.00	4287.27
*	24.000	10131.00	592.19	.33	1280.00	1740.00	2555.00	2660.00	3020.00
*	25.000	10791.00	591.92	.00	3048.37	474.90	2205.00	2293.00	3784.56
	25.000	10791.00	592.28	.36	1083.99	1805.00	2205.00	2293.00	2900.00
	26.100	11491.00	591.96	.00	2452.45	230.95	1000.00	1080.00	2736.59
	26.100	11491.00	592.36	.40	1306.94	580.00	1000.00	1080.00	1900.00
	26.110	11811.00	591.98	.00	2403.47	239.30	1000.00	1080.00	2706.50
	26.110	11811.00	592.39	.42	1304.35	580.00	1000.00	1080.00	1900.00
*	26.200	12131.00	592.32	.00	1487.72	850.00	850.00	917.00	2477.90
*	26.200	12131.00	592.44	.13	929.79	850.00	850.00	917.00	1910.00
*	26.300	12157.00	594.60	.00	1709.47	850.00	850.00	917.00	2559.47
*	26.300	12157.00	594.61	.00	1060.00	850.00	850.00	917.00	1910.00
*	26.310	12207.00	594.60	.00	934.37	216.13	624.00	688.00	1150.50
*	26.310	12207.00	594.61	.01	710.53	440.00	624.00	688.00	1150.53
1 07APR98 12:59:39									
	SECNO	CUMDS	CWSEL	DIFKWS	TOPWID	SSTA	STCHL	STCHR	ENDST
*	26.400	12692.00	594.79	.00	767.41	411.56	835.00	902.00	1178.98
	26.400	12692.00	594.81	.02	639.02	540.00	835.00	902.00	1179.02
*	27.000	13654.00	594.93	.00	1776.76	194.03	965.00	1029.00	1981.31
	27.000	13654.00	595.06	.12	1060.00	500.00	965.00	1029.00	1560.00
*	28.000	15287.00	595.20	.00	547.21	138.56	523.00	719.00	719.00
*	28.000	15287.00	595.72	.53	353.48	320.00	523.00	719.00	700.00
*	29.100	16587.00	597.18	.00	933.68	123.28	996.00	1083.00	1069.71
*	29.100	16587.00	598.10	.92	633.92	440.00	996.00	1083.00	1073.92

*	29.200	17702.00	598.29	.00	73.68	1767.68	1767.68	1841.36	1841.36
	29.200	17702.00	598.99	.70	361.36	1480.00	1767.68	1841.36	1841.36
	29.300	17782.00	598.57	.00	73.68	1767.68	1767.68	1841.36	1841.36
	29.300	17782.00	599.41	.84	73.68	1767.68	1767.68	1841.36	1841.36
	29.400	18012.00	599.79	.00	1591.42	554.14	2085.00	2150.00	2145.56
	29.400	18012.00	600.40	.61	647.78	1500.00	2085.00	2150.00	2147.78
*	30.000	18912.00	600.18	.00	1362.67	461.97	1766.00	1842.00	1824.64
*	30.000	18912.00	600.86	.68	631.16	1200.00	1766.00	1842.00	1831.16
	31.000	19424.00	600.27	.00	1996.22	478.77	2422.00	2488.00	2474.99
	31.000	19424.00	600.94	.67	680.04	1800.00	2422.00	2488.00	2480.04
	32.000	20974.00	600.48	.00	2014.88	1424.92	3380.00	3450.00	3439.80
	32.000	20974.00	601.19	.71	901.48	2541.00	3380.00	3450.00	3442.48
*	33.000	22094.00	600.70	.00	746.74	2639.24	3338.00	3392.00	3385.98
*	33.000	22094.00	601.47	.77	158.06	3229.00	3338.00	3392.00	3387.06
*	34.000	22634.00	601.14	.00	743.57	2579.19	3250.00	3332.00	3322.76
*	34.000	22634.00	601.99	.85	74.79	3250.00	3250.00	3332.00	3324.79
*	35.000	23209.00	602.22	.00	1250.65	702.89	1910.00	1958.00	1953.54
*	35.000	23209.00	603.22	1.00	525.15	1430.00	1910.00	1958.00	1955.15
*	36.000	24329.00	602.63	.00	1678.23	540.54	2182.00	2219.00	2218.76
*	36.000	24329.00	603.55	.92	938.85	1280.00	2182.00	2219.00	2218.85
	37.000	26929.00	603.04	.00	3213.33	696.08	3895.00	3911.00	4161.04
	37.000	26929.00	604.05	1.01	987.91	2900.00	3895.00	3911.00	3911.00
*	38.100	27829.00	604.60	.00	1015.34	999.66	2000.00	2049.00	2041.75
*	38.100	27829.00	605.13	.53	119.35	1900.00	2000.00	2049.00	2043.97
*	38.200	28079.00	606.93	.00	46.88	2000.00	2000.00	2047.00	2046.88
*	38.200	28079.00	607.82	.89	47.00	2000.00	2000.00	2047.00	2047.00

SECNO	CUMDS	CWSEL	DIFKWS	TOPWID	SSTA	STCHL	STCHR	ENDST
38.300	28111.00	607.78	.00	47.00	2000.00	2000.00	2047.00	2047.00
38.300	28111.00	608.67	.89	47.00	2000.00	2000.00	2047.00	2047.00

*	38.600	28191.00	609.27	.00	3498.32	976.37	2000.00	2030.00	4478.87
*	38.600	28191.00	609.47	.20	260.71	1960.00	2000.00	2030.00	2223.00
*	39.000	28891.00	609.97	.00	3109.88	82.63	994.00	1017.50	3197.61
	39.000	28891.00	610.66	.69	380.05	894.00	994.00	1017.50	1276.00
*	40.100	29201.00	610.22	.00	1102.20	1197.85	2000.00	2070.00	2321.52
*	40.100	29201.00	610.98	.76	302.74	1767.26	2000.00	2070.00	2070.00
	40.200	29301.00	610.23	.00	690.12	1440.58	2019.20	2050.70	2160.59
	40.200	29301.00	611.00	.77	250.29	1800.41	2019.20	2050.70	2050.70
*	40.300	29302.00	610.38	.00	711.74	1436.04	2019.20	2050.70	2176.31
*	40.300	29302.00	611.22	.84	370.61	1680.09	2019.20	2050.70	2050.70
	40.400	29314.00	610.43	.00	719.10	1434.49	2019.20	2050.70	2181.67
	40.400	29314.00	611.28	.85	373.66	1677.04	2019.20	2050.70	2050.70
*	40.500	29315.00	610.39	.00	720.09	1435.57	2019.20	2050.70	2177.93
*	40.500	29315.00	611.23	.84	285.24	1765.46	2019.20	2050.70	2050.70
	40.600	29385.00	610.29	.00	848.98	1291.38	2020.00	2050.00	2157.89
	40.600	29385.00	611.16	.87	125.00	1925.00	2020.00	2050.00	2050.00
	41.000	30025.00	611.33	.00	1176.78	401.28	1000.00	1075.00	1578.06
	41.000	30025.00	612.29	.96	299.70	844.14	1000.00	1075.00	1143.84
*	42.000	30825.00	613.31	.00	436.90	904.29	1000.00	1037.00	1473.33
*	42.000	30825.00	613.45	.14	199.99	1001.74	1000.00	1037.00	1236.96
	43.100	31625.00	616.86	.00	274.87	913.50	1000.00	1070.00	1188.37
*	43.100	31625.00	617.54	.69	105.00	1000.00	1000.00	1070.00	1105.00
	43.200	31725.00	617.21	.00	102.30	1002.70	1000.00	1105.00	1105.00
	43.200	31725.00	617.87	.65	104.29	1000.71	1000.00	1105.00	1105.00
	43.300	31775.00	617.22	.00	102.33	1002.67	1000.00	1105.00	1105.00
	43.300	31775.00	617.88	.65	104.31	1000.69	1000.00	1105.00	1105.00
	43.400	31975.00	617.53	.00	389.69	916.41	1000.00	1124.00	1306.10
	43.400	31975.00	618.06	.54	152.83	1000.00	1000.00	1124.00	1152.83
*	44.000	32675.00	617.83	.00	477.48	683.85	1000.00	1050.00	1161.33
*	44.000	32675.00	618.46	.63	160.71	889.29	1000.00	1050.00	1050.00
	45.000	33125.00	618.12	.00	372.90	523.45	1000.00	1042.00	1324.84

1

45.000 33125.00 618.85 .73 227.29 914.71 1000.00 1042.00 1142.00

07APR98 12:59:39

PAGE 23

SECNO	CUMDS	CWSEL	DIFKWS	TOPWID	SSTA	STCHL	STCHR	ENDST
46.000	33645.00	619.05	.00	437.71	693.00	1000.00	1047.00	1130.70
46.000	33645.00	619.57	.51	211.00	836.00	1000.00	1047.00	1047.00
*	47.000	34705.00	.00	395.73	755.98	1000.00	1127.00	1151.71
*	47.000	34705.00	.45	152.01	974.99	1000.00	1127.00	1127.00
*	48.000	35745.00	.00	85.84	990.13	1000.00	1058.00	1075.97
*	48.000	35745.00	.70	58.00	1000.00	1000.00	1058.00	1058.00
*	49.000	36395.00	.00	104.17	981.66	1012.00	1055.00	1085.84
*	49.000	36395.00	.04	62.09	1001.34	1012.00	1055.00	1063.42

1

07APR98 12:59:39

PAGE 24

SUMMARY OF ERRORS AND SPECIAL NOTES

WARNING SECNO=	14.400	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	14.400	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	15.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	15.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	17.100	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	18.200	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	18.200	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	18.400	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	18.400	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	19.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	19.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	20.100	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE

WARNING SECNO=	20.100	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	20.200	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	20.200	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	20.300	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	20.300	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	20.400	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	20.400	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	21.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	21.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	22.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	22.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	23.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	23.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	24.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	24.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	25.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
CAUTION SECNO=	26.200	PROFILE=	1	CRITICAL DEPTH ASSUMED				
CAUTION SECNO=	26.200	PROFILE=	1	PROBABLE MINIMUM SPECIFIC ENERGY				
CAUTION SECNO=	26.200	PROFILE=	1	20 TRIALS ATTEMPTED TO BALANCE WSEL				
WARNING SECNO=	26.200	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	26.300	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	26.300	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	26.310	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	26.310	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	26.400	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	27.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	28.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	28.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE

WARNING SECNO=	29.100	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	29.100	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	29.200	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	30.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	30.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	33.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	33.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	34.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	34.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	35.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	35.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	36.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	36.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
CAUTION SECNO=	38.100	PROFILE=	1	CRITICAL DEPTH ASSUMED				
CAUTION SECNO=	38.100	PROFILE=	1	PROBABLE MINIMUM SPECIFIC ENERGY				
CAUTION SECNO=	38.100	PROFILE=	1	20 TRIALS ATTEMPTED TO BALANCE WSEL				
CAUTION SECNO=	38.100	PROFILE=	2	CRITICAL DEPTH ASSUMED				
CAUTION SECNO=	38.100	PROFILE=	2	PROBABLE MINIMUM SPECIFIC ENERGY				
CAUTION SECNO=	38.100	PROFILE=	2	20 TRIALS ATTEMPTED TO BALANCE WSEL				
WARNING SECNO=	38.200	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	38.600	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	38.600	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	39.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	40.100	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	40.100	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	40.300	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	40.300	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	40.500	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	40.500	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
CAUTION SECNO=	42.000	PROFILE=	1	CRITICAL DEPTH ASSUMED				

CAUTION SECNO=	42.000	PROFILE=	1	PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION SECNO=	42.000	PROFILE=	1	20 TRIALS ATTEMPTED TO BALANCE WSEL
CAUTION SECNO=	42.000	PROFILE=	2	CRITICAL DEPTH ASSUMED
CAUTION SECNO=	42.000	PROFILE=	2	PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION SECNO=	42.000	PROFILE=	2	20 TRIALS ATTEMPTED TO BALANCE WSEL
WARNING SECNO=	43.100	PROFILE=	1	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO=	43.100	PROFILE=	2	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO=	44.000	PROFILE=	1	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO=	44.000	PROFILE=	2	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO=	47.000	PROFILE=	1	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO=	47.000	PROFILE=	2	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO=	48.000	PROFILE=	1	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO=	48.000	PROFILE=	2	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO=	49.000	PROFILE=	1	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO=	49.000	PROFILE=	2	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

1

07APR98

12:59:39

PAGE 27

Floodway width summary: MILL CREEK
Profile No. 2

Section Number	Elevation Increase	Top Width	Left		Left Sta Distance		Right Sta Distance		Right Encroach	
			Encroach Station	Center	From Center	Center Station	From Center	Center Station	Encroach Station	Center Station
14.200	1.00	73.00	1000.00		36.50	1036.50	36.50	1073.00		
14.300	.78	150.00	970.00		66.50	1036.50	83.50	1120.00		
14.400	.61	600.00	1800.00		233.50	2033.50	366.50	2400.00		
15.000	.61	360.00	770.00		255.00	1025.00	105.00	1130.00		
16.000	.58	520.00	1090.00		486.50	1576.50	33.50	1610.00		
17.100	.51	562.00	2260.00		525.00	2785.00	37.00	2822.00		
17.200	.53	460.60	2220.19		410.61	2630.80	49.99	2680.79		
17.300	.53	461.97	2218.83		411.97	2630.80	49.99	2680.79		
17.400	.57	1040.00	2050.00		997.00	3047.00	43.00	3090.00		
18.100	.53	798.00	2560.00		766.50	3326.50	31.50	3358.00		
18.200	.43	243.88	2042.00		121.94	2163.94	121.94	2285.88		
18.300	.42	243.88	2042.00		121.94	2163.94	121.94	2285.88		
18.400	.30	233.00	3162.00		53.00	3215.00	180.00	3395.00		

19.000	.18	1500.00	1900.00	425.00	2325.00	1075.00	3400.00
20.100	.21	1570.00	1000.00	827.00	1827.00	743.00	2570.00
20.200	.28	1340.00	1300.00	751.00	2051.00	589.00	2640.00
20.300	.27	1340.00	1300.00	751.00	2051.00	589.00	2640.00
20.400	.23	1300.00	1280.00	740.00	2020.00	560.00	2580.00
21.000	.24	1520.00	1620.00	1233.50	2853.50	286.50	3140.00
22.000	.23	1660.00	1200.00	1510.50	2710.50	149.50	2860.00
23.000	.24	1740.00	1160.00	1513.50	2673.50	226.50	2900.00
24.000	.33	1280.00	1740.00	867.50	2607.50	412.50	3020.00
25.000	.36	1095.00	1805.00	444.00	2249.00	651.00	2900.00
26.100	.40	1320.00	580.00	460.00	1040.00	860.00	1900.00
26.110	.42	1320.00	580.00	460.00	1040.00	860.00	1900.00
26.200	.13	1060.00	850.00	33.50	883.50	1026.50	1910.00
26.300	.00	1060.00	850.00	33.50	883.50	1026.50	1910.00
26.310	.01	1120.00	440.00	216.00	656.00	904.00	1560.00
26.400	.02	1060.00	540.00	328.50	868.50	731.50	1600.00
27.000	.12	1060.00	500.00	497.00	997.00	563.00	1560.00
28.000	.53	380.00	320.00	301.00	621.00	79.00	700.00
29.100	.92	640.00	440.00	599.50	1039.50	40.50	1080.00
29.200	.70	605.00	1480.00	324.52	1804.52	280.48	2085.00
29.300	.84	605.00	1480.00	324.52	1804.52	280.48	2085.00
29.400	.61	650.00	1500.00	617.50	2117.50	32.50	2150.00
30.000	.68	642.00	1200.00	604.00	1804.00	38.00	1842.00
31.000	.67	688.00	1800.00	655.00	2455.00	33.00	2488.00
32.000	.71	909.00	2541.00	874.00	3415.00	35.00	3450.00
33.000	.77	163.00	3229.00	136.00	3365.00	27.00	3392.00
34.000	.85	82.00	3250.00	41.00	3291.00	41.00	3332.00
35.000	1.00	528.00	1430.00	504.00	1934.00	24.00	1958.00
36.000	.92	939.00	1280.00	920.50	2200.50	18.50	2219.00
37.000	1.01	1011.00	2900.00	1003.00	3903.00	8.00	3911.00
38.100	.53	149.00	1900.00	124.50	2024.50	24.50	2049.00
38.200	.89	47.00	2000.00	23.50	2023.50	23.50	2047.00
38.300	.89	47.00	2000.00	23.50	2023.50	23.50	2047.00
38.600	.20	263.00	1960.00	55.00	2015.00	208.00	2223.00

07APR98 12:59:39

PAGE 28

39.000	.69	382.00	894.00	111.75	1005.75	270.25	1276.00
40.100	.76	302.74	1767.26	267.74	2035.00	35.00	2070.00
40.200	.77	250.29	1800.41	234.54	2034.95	15.75	2050.70
40.300	.84	370.61	1680.09	354.86	2034.95	15.75	2050.70
40.400	.85	373.67	1677.04	357.91	2034.95	15.75	2050.70
40.500	.84	285.24	1765.46	269.49	2034.95	15.75	2050.70
40.600	.87	125.00	1925.00	110.00	2035.00	15.00	2050.00
41.000	.96	299.71	844.14	193.36	1037.50	106.34	1143.84
42.000	.14	236.96	1000.00	18.50	1018.50	218.46	1236.96

07APR98

12:59:39

PAGE 29

FLOODWAY DATA, MILL CREEK
PROFILE NO. 2

STATION	----- FLOODWAY -----		WATER SURFACE ELEVATION		WATER SURFACE ELEVATION	
	WIDTH	SECTION AREA	MEAN VELOCITY	WITH FLOODWAY	WITHOUT FLOODWAY	DIFFERENCE
				AREA	VELOCITY	
14.200	73.	791.	5.6	585.5	584.5	1.0
14.300	103.	920.	4.9	585.8	585.0	.8
14.400	600.	3550.	1.3	586.3	585.7	.6
15.000	360.	1873.	2.4	586.6	586.0	.6
16.000	520.	2353.	1.9	587.1	586.5	.6
17.100	562.	2680.	1.7	587.1	586.6	.5
17.200	455.	2694.	1.7	587.2	586.7	.5
17.300	457.	2704.	1.7	587.2	586.7	.5
17.400	1036.	4289.	1.0	587.4	586.8	.6
18.100	798.	3513.	1.3	587.5	587.0	.5
18.200	213.	942.	4.7	587.8	587.4	.4
18.300	214.	949.	4.7	587.8	587.4	.4
18.400	233.	1156.	3.9	588.4	588.1	.3
19.000	1500.	4695.	1.0	590.1	589.9	.2
20.100	1570.	7164.	.6	590.3	590.1	.2
20.200	89.	771.	5.8	590.4	590.1	.3
20.300	89.	774.	5.8	590.5	590.2	.3
20.400	1300.	9265.	.5	591.1	590.9	.2
21.000	1520.	8503.	.5	591.2	591.0	.2
22.000	1660.	3316.	1.3	591.2	591.0	.2
23.000	1740.	2316.	1.9	591.5	591.3	.2
24.000	1280.	4682.	1.0	592.2	591.9	.3
25.000	1095.	5526.	.8	592.3	591.9	.4

43.100	.69	105.00	1000.00	35.00	1035.00	70.00	1105.00
43.200	.65	105.00	1000.00	52.50	1052.50	52.50	1105.00
43.300	.65	105.00	1000.00	52.50	1052.50	52.50	1105.00
43.400	.54	152.83	1000.00	62.00	1062.00	90.83	1152.83
44.000	.63	160.71	889.29	135.71	1025.00	25.00	1050.00
45.000	.73	242.00	900.00	121.00	1021.00	121.00	1142.00
46.000	.51	211.00	836.00	187.50	1023.50	23.50	1047.00
47.000	.45	152.02	974.99	88.51	1063.50	63.50	1127.00
48.000	.70	58.00	1000.00	29.00	1029.00	29.00	1058.00
49.000	.04	62.09	1001.34	32.16	1033.50	29.92	1063.42

07APR98

12:59:39

PAGE 30

FLOODWAY DATA, MILL CREEK
PROFILE NO. 2

STATION	WIDTH	SECTION AREA	----- FLOODWAY ----- MEAN VELOCITY	WATER SURFACE ELEVATION WITH FLOODWAY	WITHOUT FLOODWAY	DIFFERENCE
37.000	1011.	3162.	.9	604.0	603.0	1.0
38.100	144.	339.	8.0	605.1	604.6	.5
38.200	47.	324.	8.3	607.8	606.9	.9
38.300	47.	364.	7.4	608.7	607.8	.9
38.600	263.	1057.	2.6	609.5	609.3	.2
39.000	382.	1480.	1.8	610.7	610.0	.7
40.100	303.	1222.	2.2	611.0	610.2	.8
40.200	250.	954.	2.8	611.0	610.2	.8
40.300	371.	1100.	2.5	611.2	610.4	.8
40.400	374.	1127.	2.4	611.3	610.4	.9
40.500	285.	1098.	2.5	611.2	610.4	.8
40.600	125.	676.	4.0	611.2	610.3	.9
41.000	300.	1201.	2.3	612.3	611.3	1.0
42.000	235.	410.	6.6	613.4	613.3	.1

26.100	1320.	6118.	.7	592.4	592.0	.4
26.110	1320.	5695.	.8	592.4	592.0	.4
26.200	1060.	3497.	1.3	592.4	592.3	.1
26.300	1060.	5705.	.8	594.6	594.6	.0
26.310	711.	3556.	1.3	594.6	594.6	.0
26.400	639.	3426.	1.3	594.8	594.8	.0
27.000	1060.	4823.	.9	595.0	594.9	.1
28.000	380.	1233.	3.6	595.7	595.2	.5
29.100	634.	2499.	1.8	598.1	597.2	.9
29.200	361.	1535.	2.9	599.0	598.3	.7
29.300	74.	684.	6.5	599.4	598.6	.8
29.400	648.	2174.	2.1	600.4	599.8	.6
30.000	631.	2956.	.9	600.9	600.2	.7
31.000	680.	2775.	1.0	601.0	600.3	.7
32.000	901.	3394.	.8	601.2	600.5	.7
33.000	158.	1001.	2.7	601.5	600.7	.8
34.000	75.	478.	5.7	602.0	601.1	.9
35.000	525.	1894.	1.4	603.2	602.2	1.0
36.000	939.	3496.	.8	603.5	602.6	.9

43.100	105.	612.	4.4	617.6	616.9	.7
43.200	104.	766.	3.5	617.9	617.2	.7
43.300	104.	767.	3.5	617.9	617.2	.7
43.400	153.	1003.	2.7	618.0	617.5	.5
44.000	161.	879.	3.1	618.4	617.8	.6
45.000	227.	770.	3.5	618.8	618.1	.7
46.000	211.	615.	4.4	619.6	619.1	.5
47.000	152.	875.	3.1	621.5	621.0	.5
48.000	58.	288.	9.4	622.9	622.2	.7
49.000	62.	383.	7.1	628.1	628.1	.0

* HEC-2 WATER SURFACE PROFILES *
* *
* Version 4.6.2; May 1991 *
* *
* RUN DATE 03AUG07 TIME 09:10:09 *

* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET, SUITE D *
* DAVIS, CALIFORNIA 95616-4687 *
* (916) 756-1104 *

```

X X XXXXXXXX XXXX
X X X X X
X X X X X
XXXXXXX XXXX
X X X X X
X X X X X
X X XXXXXXXX XXXX

```

03AUG07 09:10:09

PAGE 1

HEC-2 WATER SURFACE PROFILES

Version 4.6.2; May 1991

T1 Flood Insurance Study-Butler County, Ohio (EMW-93-4106)
T2 100-Year Flood
T3 MILL CREEK
T4 Duplicate Effective Model 07-05-07

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
	0	4							584.46	
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE
	1	0								

THIS RUN EXECUTED 03AUG07 09:10:09

J3 VARIABLE CODES FOR SUMMARY PRINTOUT

115	200	38	66	1	52	4	53	21	22
54									

J5 LPRNT NUMSEC *****REQUESTED SECTION NUMBERS*****

-10
 Starting WSL from MCSHARON.HC2 output
 Peak flow from MLL95.HC1 SR747D
 Current effective floodway will be maintained

QT	5	2467	4009	4465	6335	4465			
NC	0.08	0.08	0.04	0.3	0.5				
ET						9.1	1000	1073	1000

Section modified by Crescent Park
 Section revised based on 9/93 Signco drainage plan by Neyer
 and 2/26/94 bridge survey

X1	14.2	17	1000	1073	100	100.0			
X3	10								
GR	586	350	585	795	585	900	584	585.5	586.4
GR	581.8	1000	575.4	1022	572	1027	569.7	950	583
GR	570	1031.4	570	1031.5	572	1050	577.5	1030	569.7
GR	582	1680	586	1740				1073	582.9
									1105

ET CRESCENTVILLE ROAD BRIDGE

Top of Road 585.6, low chord 584.5
 Bridge area west of west pier ineffective flow

SB	1.00	1.56	2.80	0.00	19.0	2.00	657	1.91	570
----	------	------	------	------	------	------	-----	------	-----

1 03AUG07 09:10:09 PAGE 2

X1	14.3		1	584.5	32.0	32.0			
X2					585.6				
X3	10								
BT	-6	350	586		795	585.6			586.4
BT		1073	586.4		1340	588			585.6
									590
NH	7	1.0	1710	0.06	1988	0.13	2000	0.04	2067
NH	2100	0.08	2200	1.0	2630				0.13

Butler County Section A
 Section 14.4 2/26/94 survey section u/s of Crescentville Rd
 ET field 9 and 10 original FIS floodway, field 7, 8 optimized floodway

ET	14.4	18	2000	2067	120	9.1	1800	2200	1800	2400
X1	14.4	18	2000	2067	120	120	1800	2200	1800	2400
GR	588	750	585.8	925	585.1	1050	581.8	1150	583.2	1200
GR	581.4	1710	579.2	1988	580.8	2000	572	2001	570.9	2002
GR	570.2	2030	573.2	2032	577.6	2040	579.7	2067	582	2100
GR	582	2200	582	2590	589	2630				
NC	0.08	0.12	0.04	0.1	0.3					
NH	7	1.0	713	0.06	984	0.13	1000	0.04	1050	0.13
NH	1076	0.12	1131	1.0	1757					

ET	17.1	16	2748	2822	150	9.1	2400	2822	2260	2822
X1	0592	0	590	230	588	100	100			
GR	584	1625	584	2400	582	550	588	1070	586	1476
GR	573.7	2797	586	2822	590	2730	582	2748	573.7	2764
GR	595.8	3001				2832	594	2850	595.6	3000

NC 0.10 0.12 0.055 0.3 0.5
 Section taken off Windisch Road bridge plan 3/84
 Sheet 18/34, "Channel Sections, Mill Creek" Station 5+00

ET	17.2	20	3000	3118.6	300	10.4	2400	2822	2160	2822
X1	10					130	140	0.843	0.	
X3								584.5	592.0	
GR	592.0	330	590.0	760	589	1000	587.8	1330	585.5	1885
GR	584	2227	583.8	2327	583.6	2427	583.4	2527	583.2	2627
GR	583.0	2727	582.5	2877	581.5	2927	581.5	2977	582	3000
GR	573.6	3026.9	573.6	3082.7	590	3118.6	593.5	3168.6	594.8	3218.6

Windisch Road (BUT-CR 127-0.40)
 Bridge data obtained from Butler County Bridge Plan Project BRZ-0905(1), 1984.
 Top of Road 592, Low chord 589
 1959 highway mark from COE report 585.5 approximately

ET	1.25	1.5	2.0	0	45	10.4	2600	2680	2160	3100
SB				0.3	0.5	2	976	2	573.6	
NC										
X1	17.3	20	3000	3118.6	34	34		0.843		
X2			1	587.8	584.5				0.843	
X3	10							584.5	592	
BT	-20	330	592		760	590		1000	589	
BT		1330	588		1885	586		2227	585.2	
BT		2327	585.1		2427	585		2527	585	
BT		2627	584.8		2727	584.5		2877	585.2	
BT		2927	586.1		2977	587.2		3000	587.9	
BT		3026.9	588.7		3082.7	590.4		3118.6	592.0	
BT		3168.6	593.5		3218.6	594.8				
GR	592.0	330	590.0	760	589	1000	587.8	1330	585.5	1885
GR	584	2227	583.8	2327	583.6	2427	583.4	2527	583.2	2627
GR	583.0	2727	582.5	2877	581.5	2927	581.5	2977	582	3000
GR	573.6	3026.9	573.6	3082.7	590	3118.6	593.5	3168.6	594.8	3218.6

NH 4 1.0 2510 0.06 3013 0.04 3059 0.13 3126

left bank elevation was changed from 588 to 583
 Cross Section revised using Butler County bridge plans for Windisch Road,
 Channel Station 7+00, and Butler County 1980 Topos (1-26-94).

Section E
 ET
 X1 17.4 12 3000 3094 150 9.1 2700 3094 2050 3090 183

GR	592	430	590	590	588	1470	586	1993	584	2510
GR	583	2864	581.8	2910	580.0	3000	573.8	3013	573.8	3059
GR	591.5	3094	600	3126						

1 03AUG07 09:10:09 PAGE 4

NC				0.1	0.3					
NH	5	1.0	2690	0.06	3220	0.13	3295	0.04	3358	0.13
NH	3500									

1 03AUG07 09:10:09 PAGE 5

T1 Flood Insurance Study EMW-93-C-4106
T2 Floodway Analysis
T3 Mill Creek Butler County, Ohio
T4 Duplicate Effective Model 07-05-07

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
	0	6							585.46	
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE

2 -1

1 03AUG07 09:10:09 PAGE 6

THIS RUN EXECUTED 03AUG07 09:10:09

HEC-2 WATER SURFACE PROFILES
Version 4.6.2; May 1991

NOTE- ASTERISK (*) AT LEFT OF CROSS-SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST

MILL CREEK

SUMMARY PRINTOUT

SECNO	CUMDS	CWSEL	DIFKWS	TOPWID	SSTA	STCHL	STCHR	ENDST
14.200	.00	584.46	.00	73.00	1000.00	1000.00	1073.00	1073.00
14.200	.00	585.46	1.00	73.00	1000.00	1000.00	1073.00	1073.00
14.300	32.00	585.04	.00	73.00	1000.00	1000.00	1073.00	1073.00
14.300	32.00	585.82	.78	103.00	970.00	1000.00	1073.00	1073.00
* 14.400	152.00	585.73	.00	1673.60	937.71	2000.00	2067.00	2611.31
* 14.400	152.00	586.33	.61	600.00	1800.00	2000.00	2067.00	2400.00
* 15.000	992.00	585.98	.00	1160.12	198.86	1000.00	1050.00	1421.81
* 15.000	992.00	586.59	.61	360.00	770.00	1000.00	1050.00	1130.00
16.000	1642.00	586.50	.00	1109.08	515.95	1543.00	1610.00	1625.03
16.000	1642.00	587.08	.58	520.00	1090.00	1543.00	1610.00	1610.00
* 17.100	1742.00	586.64	.00	1478.20	1345.41	2748.00	2822.00	2823.61
17.100	1742.00	587.15	.51	562.00	2260.00	2748.00	2822.00	2822.00
17.200	1882.00	586.70	.00	1278.96	1395.75	2580.81	2680.79	2674.71
17.200	1882.00	587.23	.53	455.49	2220.19	2580.81	2680.79	2675.68
17.300	1916.00	586.71	.00	1280.85	1393.88	2580.81	2680.79	2674.73
17.300	1916.00	587.24	.53	456.88	2218.83	2580.81	2680.79	2675.70
17.400	2099.00	586.82	.00	1306.29	1778.46	3000.00	3094.00	3084.75
17.400	2099.00	587.39	.57	1035.88	2050.00	3000.00	3094.00	3085.88

1

03AUG07 09:10:09

PAGE 7

SUMMARY OF ERRORS AND SPECIAL NOTES

WARNING SECNO=	14.400	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	14.400	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	15.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	15.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE

WARNING SECNO= 17.100 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

1

03AUG07 09:10:09

PAGE 8

Floodway width summary: MILL CREEK
Profile No. 2

Section Number	Elevation Increase	Top Width	Left Encroach Station	Left Sta Distance From Center	Center Station	Right Sta Distance From Center	Right Encroach Station
14.200	1.00	73.00	1000.00	36.50	1036.50	36.50	1073.00
14.300	.78	150.00	970.00	66.50	1036.50	83.50	1120.00
14.400	.61	600.00	1800.00	233.50	2033.50	366.50	2400.00
15.000	.61	360.00	770.00	255.00	1025.00	105.00	1130.00
16.000	.58	520.00	1090.00	486.50	1576.50	33.50	1610.00
17.100	.51	562.00	2260.00	525.00	2785.00	37.00	2822.00
17.200	.53	460.60	2220.19	410.61	2630.80	49.99	2680.79
17.300	.53	461.97	2218.83	411.97	2630.80	49.99	2680.79
17.400	.57	1040.00	2050.00	997.00	3047.00	43.00	3090.00

1

03AUG07 09:10:09

PAGE 9

FLOODWAY DATA, MILL CREEK
PROFILE NO. 2

STATION	WIDTH	FLOODWAY SECTION AREA	MEAN VELOCITY	WATER SURFACE ELEVATION WITH FLOODWAY	WITHOUT DIFFERENCE FLOODWAY
14.200	73.	791.	5.6	585.5	584.5
14.300	103.	920.	4.9	585.8	585.0
14.400	600.	3550.	1.3	586.3	585.7
15.000	360.	1873.	2.4	586.6	586.0
16.000	520.	2353.	1.9	587.1	586.5
17.100	562.	2680.	1.7	587.1	586.6
17.200	455.	2694.	1.7	587.2	586.7
17.300	457.	2704.	1.7	587.2	586.7
17.400	1036.	4289.	1.0	587.4	586.8

HEC-2 WATER SURFACE PROFILES
Version 4.6.2; May 1991

T1 Flood Insurance Study-Butler County, Ohio (EMW-93-4106)
T2 100-Year Flood
T3 MILL CREEK
T4 Revised Existing Conditions Model 07-05-07

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
0		4							584.46	
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE
1	0	0	-1							

J3 VARIABLE CODES FOR SUMMARY PRINTOUT

115	200	38	66	1	52	4	53	21	22
54									

J5 LPRNT NUMSEC *****REQUESTED SECTION NUMBERS*****

-10 -10
Starting WSL from MCSHARON.HC2 output
Peak flow from MILL95.HC1 SR747D
Current effective floodway will be maintained

QT	5	2467	4009	4465	6335	4465				
NC	0.08	0.08	0.04	0.3	0.5					
ET					9.1	1000	1073	1000	1073	1073
Section modified by Crescent Park										
Section revised based on 9/93 Signco drainage plan by Neyer										
and 2/26/94 bridge survey										
X1	14.2	17	1000	1073	100	100.0				
X3	10									
GR	586	350	585	795	585	900	584	586.4	583	970
GR	581.8	1000	575.4	1022	572	1027	569.7	586.4	583	970
GR	570	1031.4	570	1031.5	572	1050	577.5	586.4	583	970
GR	582	1680	586	1740				586.4	583	970
ET										
CRESCENTVILLE ROAD BRIDGE										
Top of Road 585.6, low chord 584.5										
Bridge area west of west pier ineffective flow										
SB	1.00	1.56	2.80	0.00	19.0	2.00	657	1.91	570	570

X1	14.3		1	584.5	32.0	32.0	32.0		
X2				585.6					
X3	10		586	795					586.4
BT	-6		586.4	1340	585.6				585.6
BT					588				1000
NH	7	1.0	1710	0.06					1540
NH	2100	0.08	2200	1.0					590
Butler County Section A									
Section 14.4 2/26/94 survey section u/s of Crescentville Rd									
ET field 9 and 10 original FIS floodway, field 7, 8 optimized floodway									
ET					9.1				
X1	14.4	18	2000	2067			1800		2400
GR	588	750	585.8	925	120		120		1200
GR	581.4	1710	579.2	1988	585.1		581.8		583.2
GR	570.2	2030	573.2	2032	580.8		572		2002
GR	582	2200	582	2590	577.6		579.7		2100
NC	0.08	0.12	0.04	0.1	589				
NH	7	1.0	770	0.06					0.13
NH	1076	0.12	1131	1.0	1757				
Butler County Section B									
Section 15.0 4/94 survey section CM-1									
ET					9.1		770		1130
X1	15.0	40	1000	1050	800		840		331
GR	588.80	192	586.99	196	584.53		584.05		930
GR	583.14	437	582.85	505	581.98		582.00		1000
GR	581.96	971	581.11	976	581.99		582.22		1017
GR	573.51	1002	571.58	1003	570.18		570.6		1040
GR	572.68	1025	573.55	1025	575.49		579.35		1070
GR	579.26	1045	580.52	1050	582.5		581.39		1131
GR	583.19	1076	583.19	1082	582.52		583.21		1757
GR	586.15	1271	585.83	1364	586.09		587.62		0.04
NH	6	1.0	910	1.0	1515		1728		
NH	1892	0.13	1950						
Section B2 - New Section For Development East of Windisch Road									
Section 15.5 Existing Conditions 07/2007 BB Field Topo Merged w/Aerial Topo									
ET					9.1		365		1927
X1	15.5	22	1844	1892	346				
X3	10								
GR	588.00	927	586.00	946	585.00		584.00		1382
GR	582.00	1410	581.00	1436	580.00		579.50		1538
GR	579.14	1602	580.00	1641	581.00		582.00		1728
GR	580.00	1844	571.85	1855	571.85		585.00		1908
GR	589.80	1927	590.00	1950					
NH	5	1.0	1090	0.06	1530		1543		0.13
NH	1770								
Butler County Section C, old section D									
Original HEC-2 data with both floodplains modified using 1980 topo									
Channel area replaced by Windisch Road bridge plan sheet 3/84									
Sheet 17/34 "Channel Sections" station 2+50									
Existing Conditions 07/2007 BB Field Topo Merged w/Aerial Topo									

[illegible]

Butler County Section D, old section E
Section E is modified to exclude East Fork Mill Creek
Section extended 1000' to the north (left bank)
Channel area replaced by Windisch Road bridge plan, sheet 3/84
Sheet 17/34 "Channel Sections" station 3+50

ET	17.1	26	2748	2822	150	9.1	2400	2822	2260	2822
X1	17.1	26	2748	2822	150	100	100			
X3	10									
GR	0592	0	590	230	588	550	588	582.5	586	1476
GR	587.5	1610	587	1764	588	1787	588	1070	585	1900
GR	584.5	1995	585	2075	584.5	2260	584	2285	583	2405
GR	577	2425	577	2600	582.0	2630	583.5	2748	573.7	2764
GR	573.7	2797	586	2822	590	2832	594	2850	595.6	3000
GR	595.8	3001								

NC 0.10 0.12 0.055 0.3 0.5

Section taken off Windisch Road bridge plan 3/84

Sheet 18/34, "Channel Sections, Mill Creek" Station 5+00

ET	17.2	20	3000	3118.6	300	10.4	2400	2822	2160	3100
X1					300	130	140	0.843	0.	
X3	10							584.5	592.0	
GR	592.0	330	590.0	760	589	1000	587.8	1330	585.5	1885
GR	584	2227	583.8	2327	583.6	2427	583.4	2527	583.2	2627
GR	583.0	2727	582.5	2877	581.5	2927	581.5	2977	582	3000
BP	573.6	3026.9	573.6	3082.7	590	3118.6	593.5	3168.6	594.8	3218.6

Windisch Road (BUT-CR 127-0.40)
 Bridge data obtained from Butler County Bridge Plan Protect BRZ-0905(1), 1984.

Top of Road 592, Low chord 589
1959 highwater mark from COE report 585.5 approximately

ET	1.25	1.5	2.0	0	45	10.4	2600	2680	2160	3100
SB	1.25	1.5	2.0	0	45	2	976	2	573.6	573.6
NC				0.3	0.5					
X1	17.3	20	3000	3118.6	34	34		0.843		
X2			1	587.8	584.5				0.843	
X3	10							584.5	592	
BT	-20	330	592		760	590		1000	589	
BT		1330	588		1885	586			585.2	
BT		2327	585.1		2427	585		2527	585	
BT		2627	584.8		2727	584.5		2877	585.2	
BT		2927	586.1		2977	587.2		3000	587.9	
BT		3026.9	588.7		3082.7	590.4		3118.6	592.0	

BT	3168.6	593.5	3218.6	594.8								
GR	592.0	330	590.0	589	1000				587.8	1330	585.5	1885
GR	584	2227	583.8	583.6	2427				583.4	2527	583.2	2627
GR	583.0	2727	582.5	581.5	2927				581.5	2977	582	3000
GR	573.6	3026.9	573.6	590	3118.6				593.5	3168.6	594.8	3218.6

NH 4 1.0 2510 0.06 3013 0.04 3059 0.13 3126
left bank elevation was changed from 588 to 583
Cross Section revised using Butler County bridge plans for Windisch Road,
Channel Station 7+00, and Butler County 1980 Topos (1-26-94).

Section E

ET					9.1	2700	3094	2050	3090
X1	17.4	12	3000	3094	210	183			
GR	592	430	590	590	1470	586	1993	584	2510
GR	583	2864	581.8	2910	3000	573.8	3013	573.8	3059
GR	591.5	3094	600	3126					

NC				0.3					
NH 5		1.0	2690	0.06	0.13	3295	0.04	3358	0.13
NH 3500									

T1 Flood Insurance Study EMW-93-C-4106
T2 Floodway Analysis
T3 Mill Creek Butler County, Ohio
T4 Corrected Effective Model 07-05-07

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
0		6							585.46	
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE

HEC-2 WATER SURFACE PROFILES

Version 4.6.2; May 1991

NOTE- ASTERISK (*) AT LEFT OF CROSS-SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST

MILL CREEK

SUMMARY PRINTOUT

SECNO	CUMDS	CWSEL	DIFKWS	TOPWID	SSTA	STCHL	STCHR	ENDST
14.200	.00	584.46	.00	73.00	1000.00	1000.00	1073.00	1073.00
14.200	.00	585.46	1.00	73.00	1000.00	1000.00	1073.00	1073.00
14.300	32.00	585.04	.00	73.00	1000.00	1000.00	1073.00	1073.00
14.300	32.00	585.82	.78	103.00	970.00	1000.00	1073.00	1073.00
14.400	152.00	585.73	.00	1673.60	937.71	2000.00	2067.00	2611.31
14.400	152.00	586.33	.61	600.00	1800.00	2000.00	2067.00	2400.00
15.000	992.00	585.98	.00	1160.12	198.86	1000.00	1050.00	1421.81
15.000	992.00	586.59	.61	360.00	770.00	1000.00	1050.00	1130.00
15.500	1357.00	586.27	.00	953.61	943.46	1844.00	1892.00	1897.07
15.500	1357.00	586.88	.61	384.52	1515.00	1844.00	1892.00	1899.52
16.000	1652.00	586.40	.00	1087.30	536.69	1543.00	1610.00	1623.99
16.000	1652.00	586.99	.60	520.00	1090.00	1543.00	1610.00	1610.00
17.100	1752.00	586.43	.00	1064.16	1387.99	2748.00	2822.00	2823.08
17.100	1752.00	587.02	.59	562.00	2260.00	2748.00	2822.00	2822.00
17.200	1892.00	586.46	.00	1228.69	1445.56	2580.81	2680.79	2674.26
17.200	1892.00	587.03	.57	417.03	2258.28	2580.81	2680.79	2675.31
17.300	1926.00	586.48	.00	1233.63	1440.67	2580.81	2680.79	2674.30
17.300	1926.00	587.05	.57	421.05	2254.30	2580.81	2680.79	2675.35
17.400	2109.00	586.61	.00	1250.28	1834.05	3000.00	3094.00	3084.33
17.400	2109.00	587.23	.62	1035.55	2050.00	3000.00	3094.00	3085.55

SUMMARY OF ERRORS AND SPECIAL NOTES

WARNING SECNO=	14.400	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE RANGE
WARNING SECNO=	14.400	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE RANGE
WARNING SECNO=	15.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE RANGE
WARNING SECNO=	15.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE RANGE
WARNING SECNO=	15.500	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE RANGE
WARNING SECNO=	16.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE RANGE
WARNING SECNO=	17.200	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE RANGE
WARNING SECNO=	17.200	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE RANGE

Floodway width summary: MILL CREEK

Profile No. 2

Section Number	Elevation Increase	Top Width	Left Encroachment Station	Left Sta		Center Station	Right Sta		Right Encroachment Station
				Distance From Center	Distance To Center		Distance From Center	Distance To Center	
14.200	1.00	73.00	1000.00	36.50	1036.50	36.50	1073.00		
14.300	.78	150.00	970.00	66.50	1036.50	83.50	1120.00		
14.400	.61	600.00	1800.00	233.50	2033.50	366.50	2400.00		
15.000	.61	360.00	770.00	255.00	1025.00	105.00	1130.00		
15.500	.61	412.00	1515.00	353.00	1868.00	59.00	1927.00		
16.000	.60	520.00	1090.00	486.50	1576.50	33.50	1610.00		
17.100	.59	562.00	2260.00	525.00	2785.00	37.00	2822.00		
17.200	.57	422.51	2258.28	372.52	2630.80	49.99	2680.79		
17.300	.57	426.49	2254.30	376.50	2630.80	49.99	2680.79		
17.400	.62	1040.00	2050.00	997.00	3047.00	43.00	3090.00		

FLOODWAY DATA, MILL CREEK
PROFILE NO. 2

STATION	WIDTH	FLOODWAY SECTION AREA	FLOODWAY MEAN VELOCITY		WATER SURFACE ELEVATION	
			WITH FLOODWAY	WITHOUT FLOODWAY	DIFFERENCE	
14.200	73.	791.	5.6	585.5	584.5	1.0
14.300	103.	920.	4.9	585.8	585.0	.8
14.400	600.	3550.	1.3	586.3	585.7	.6
15.000	360.	1873.	2.4	586.6	586.0	.6
15.500	385.	2667.	1.7	586.9	586.3	.6
16.000	520.	3524.	1.3	587.0	586.4	.6
17.100	562.	3870.	1.2	587.0	586.4	.6
17.200	417.	2458.	1.8	587.1	586.5	.6
17.300	421.	2483.	1.8	587.1	586.5	.6
17.400	1036.	4117.	1.1	587.2	586.6	.6

HEC-2 WATER SURFACE PROFILES

Version 4.6.2; May 1991

T1 Flood Insurance Study-Butler County, Ohio (EMW-93-4106)
T2 100-Year Flood
T3 MILL CREEK
T4 Revised Proposed Conditions Model 10-19-07

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
0		4							584.46	

CHNIM ITRACE

1 0 -1

J3 VARIABLE CODES FOR SUMMARY PRINTOUT

115	200	38	66	1	52	4	53	21	22
54									

J5 LPRNT NUMSEC *****REQUESTED SECTION NUMBERS*****

-10 -10
Starting WSL from MCSHARON.HC2 output
Peak flow from MILL95.HC1 SR747D
Current effective floodway will be maintained

QT	5	2467	4009	4465	6335	4465				
NC	0.08	0.08	0.04	0.3	0.5	9.1	1000	1073	1000	1073
ET										

Section modified by Crescent Park
Section revised based on 9/93 Signco drainage plan by Neyer
and 2/26/94 bridge survey

X1	14.2	17	1000	1073	100	100.0	100.0			
X3	10							585.5	586.4	
GR	586	350	585	795	585	900	584	950	583	970
GR	581.8	1000	575.4	1022	572	1027	569.7	1030	569.7	1030.1
GR	570	1031.4	570	1031.5	572	1050	577.5	1073	582.9	1105
GR	582	1680	586	1740						

ET					9.1	970	1120	970	1120	
----	--	--	--	--	-----	-----	------	-----	------	--

Revised
Proposed

CRESCENTVILLE ROAD BRIDGE

Top of Road 585.6, low chord 584.5

Bridge area west of west pier ineffective flow

SB	1.00	1.56	2.80	0.00	19.0	2.00	657	1.91	570	570
X1	14.3				32.0	32.0	32.0			
X2			1	584.5	585.6					
X3	10							585.6	586.4	
BT	-6	350	586		795	585.6		1000	585.6	
BT		1073	586.4		1340	588		1540	590	

NH	7	1.0	1710	0.06	1988	0.13	2000	0.04	2067	0.13
NH	2100	0.08	2200	1.0	2630					

Butler County Section A

Section 14.4 2/26/94 survey section u/s of Crescentville Rd

ET field 9 and 10 original FIS floodway, field 7, 8 optimized floodway

ET					9.1	1800	2200	1800	2400	
X1	14.4	18	2000	2067	120	120				
GR	588	750	585.8	925	585.1	581.8	1150	583.2	1200	
GR	581.4	1710	579.2	1988	580.8	572	2001	570.9	2002	
GR	570.2	2030	573.2	2032	577.6	579.7	2067	582	2100	
GR	582	2200	582	2590	589	2630				

NC	0.08	0.12	0.04	0.1	0.3					
NH	7	1.0	770	0.06	984	1000	0.04	1050	0.13	
NH	1076	0.12	1131	1.0	1757					

Butler County Section B

Section 15.0 4/94 survey section CM-1

ET					9.1	770	1130	770	1130	
X1	15.0	40	1000	1050	800	840				
GR	588.80	192	586.99	196	584.53	584.05	240	583.89	331	
GR	583.14	437	582.85	505	581.98	582.00	814	582.66	930	
GR	581.96	971	581.11	976	581.99	582.22	994	580.75	1000	
GR	573.51	1002	571.58	1003	570.18	570.6	1011	571.97	1017	
GR	572.68	1022	573.55	1025	575.49	579.35	1034	579.03	1040	
GR	579.26	1045	580.52	1050	582.5	581.39	1067	582.06	1070	
GR	583.19	1076	583.19	1082	582.52	583.21	1098	584.53	1131	
GR	586.15	1271	585.83	1364	586.09	587.62	1611	588.48	1757	

NH	6	1.0	910	1.0	1515	1782	0.13	1844	0.04	
NH	1892	0.13	1950							

Section B2 - New Section For Development East of Windisch Road

Section 15.5 Existing Conditions 07/2007 BB Field Topo Merged w/Aerial Topo

ET	15.5	22	1844	1892	346	9.1	334	365	1515	1927
X1	10									
X3		946	583.20	979	584.00	1018		585.00	585.80	1125
GR	586.00	1126	589.80	1408	589.00	1420		587.75	587.75	1507
GR	589.80	1580	580.00	1641	579.00	1770		579.00	582.00	1782
GR	578.80	1844	571.85	1855	571.85	1877		585.00	589.00	1908
GR	580.00	1927	590.00	1950						
GR	589.80									

NH	5	1.0	1090	0.06	1530	0.13		1543	1610	0.13
----	---	-----	------	------	------	------	--	------	------	------

NH 1770

Butler County Section C, old section D

Original HEC-2 data with both floodplains modified using 1980 topo

Channel area replaced by Windisch Road bridge plan sheet 3/84

Sheet 17/34 "Channel Sections" station 2+50

Existing Conditions 07/2007 BB Field Topo Merged w/Aerial Topo

ET	16.0	22	1543	1610	290	9.1	290	1200	1090	1610
X1	10									
X3		480	586	485	585	594		585.8	589.8	723
GR	588.0	987	589	1000	588	1047		579	578	1090
GR	589.8	1332	578	1495	582	1507		581	577	1550
GR	577.4	1559	573.2	1572	583	1597		585	590	1660
GR	573.2	1678	595.3	1770						
GR	595.0									

NH	5	1.0	1625	0.06	2730	0.13		2748	2822	0.13
----	---	-----	------	------	------	------	--	------	------	------

NH 3001

Butler County Section D, old section E

Section E is modified to exclude East Fork Mill Creek

Section extended 1000' to the north (left bank)

Channel area replaced by Windisch Road bridge plan sheet 3/84

Sheet 17/34 "Channel Sections" station 3+50

ET	17.1	26	2748	2822	150	9.1	100	2400	2260	2822
X1	10									
X3		0	590	230	588	550		588	586.0	1476
GR	0592	1610	587	1764	588	1787		589.8	586	1895
GR	587.5	2016	589.8	2029	585	2133		586.0	589	2232
GR	589.0	2250	578	2256	577	2411		583.5	573.7	2764
GR	581.0	2797	586	2822	590	2832		594	595.6	3000
GR	573.7	3001								
GR	595.8									

NC	0.10	0.12	0.055	0.3	0.5					
----	------	------	-------	-----	-----	--	--	--	--	--

ET	17.2	20	3000	3118.6	300	10.4	2400	2822	2160	3100
X1	10				300	130	140	0.843	0.	
X3								584.5	592.0	
GR	592.0	330	590.0	760	589	1000	587.8	1330	585.5	1885
GR	584	2227	583.8	2327	583.6	2427	583.4	2527	583.2	2627
GR	583.0	2727	582.5	2877	581.5	2927	581.5	2977	582	3000
GR	573.6	3026.9	573.6	3082.7	590	3118.6	593.5	3168.6	594.8	3218.6

Top of Road 592, Low chord 589
1959 highwater mark from COE report 585.5 approximately

ET	1.25	1.5	2.0	0	45	10.4	2600	2680	2160	3100
SB						2	976	2	573.6	573.6
NC				0.3	0.5					
X1	17.3	20	3000	3118.6	34	34		0.843	0.843	
X2			1	587.8	584.5					
X3	10									
BT	-20	330	592	760	760	590		584.5	592	
BT		1330	588	1885	1885	586		1000	589	
BT		2327	585.1	2427	2427	585		2227	585.2	
BT		2627	584.8	2727	2727	584.5		2527	585	
BT		2927	586.1	2977	2977	587.2		2877	585.2	
BT		3026.9	588.7	3082.7	3082.7	590.4		3000	587.9	
BT		3168.6	593.5	3218.6	3218.6	594.8		3118.6	592.0	
GR	592.0	330	590.0	760	589	1000	587.8	1330	585.5	1885
GR	584	2227	583.8	2327	583.6	2427	583.4	2527	583.2	2627
GR	583.0	2727	582.5	2877	581.5	2927	581.5	2977	582	3000
GR	573.6	3026.9	573.6	3082.7	590	3118.6	593.5	3168.6	594.8	3218.6
NH	4	1.0	2510	0.06	3013	0.04	3059	0.13	3126	

left bank elevation was changed from 588 to 583

Cross Section revised using Butler County bridge plans for Windisch Road,
Channel Station 7+00, and Butler County 1980 Topos (1-26-94).

Section E												
ET								9.1	2700	3094	2050	3090
X1	17.4	12	3000	3094	150	210	183					
GR	592	430	590	590	588	1470	586			1993	584	2510
GR	583	2864	581.8	2910	580.0	3000	573.8			3013	573.8	3059
GR	591.5	3094	600	3126								
NC				0.1	0.3							
NH	5	1.0	2690	0.06	3220			0.13	3295	0.04	3358	0.13
NH	3500											
T1	Flood Insurance Study EMW-93-C-4106											
T2	Floodway Analysis											
T3	Butler County, Ohio											
T4	Proposed Conditions Model 07-05-07											
J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ		
	0	6							585.46			
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE		

HEC-2 WATER SURFACE PROFILES

Version 4.6.2; May 1991

NOTE- ASTERISK (*) AT LEFT OF CROSS-SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST

MILL CREEK

SUMMARY PRINTOUT

SECNO	CUMDS	CWSEL	DIFKWS	TOPWID	SSTA	STCHL	STCHR	ENDST
14.200	.00	584.46	.00	73.00	1000.00	1000.00	1073.00	1073.00
14.200	.00	585.46	1.00	73.00	1000.00	1000.00	1073.00	1073.00
14.300	32.00	585.04	.00	73.00	1000.00	1000.00	1073.00	1073.00
14.300	32.00	585.82	.78	103.00	970.00	1000.00	1073.00	1073.00
14.400	152.00	585.73	.00	1673.60	937.71	2000.00	2067.00	2611.31
14.400	152.00	586.33	.61	600.00	1800.00	2000.00	2067.00	2400.00
15.000	992.00	585.98	.00	1160.12	198.86	1000.00	1050.00	1421.81
15.000	992.00	586.59	.61	360.00	770.00	1000.00	1050.00	1130.00
15.500	1357.00	586.26	.00	556.99	946.00	1844.00	1892.00	1897.03
15.500	1357.00	586.88	.62	384.51	1515.00	1844.00	1892.00	1899.51
16.000	1652.00	586.37	.00	808.82	484.07	1543.00	1610.00	1623.72
16.000	1652.00	586.97	.60	520.00	1090.00	1543.00	1610.00	1610.00
17.100	1752.00	586.38	.00	833.68	1397.10	2748.00	2822.00	2822.97
17.100	1752.00	586.98	.60	562.00	2260.00	2748.00	2822.00	2822.00
17.200	1892.00	586.42	.00	1218.77	1455.40	2580.81	2680.79	2674.17
17.200	1892.00	586.99	.57	410.02	2265.22	2580.81	2680.79	2675.24
17.300	1926.00	586.45	.00	1225.84	1448.40	2580.81	2680.79	2674.23
17.300	1926.00	587.02	.57	414.86	2260.44	2580.81	2680.79	2675.29
17.400	2109.00	586.57	.00	1241.11	1843.15	3000.00	3094.00	3084.26
17.400	2109.00	587.20	.63	1035.49	2050.00	3000.00	3094.00	3085.49

SUMMARY OF ERRORS AND SPECIAL NOTES

WARNING SECNO=	14.400	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	14.400	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	15.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	15.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	15.500	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	15.500	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	16.000	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	16.000	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	17.200	PROFILE=	1	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE
WARNING SECNO=	17.200	PROFILE=	2	CONVEYANCE	CHANGE	OUTSIDE	ACCEPTABLE	RANGE

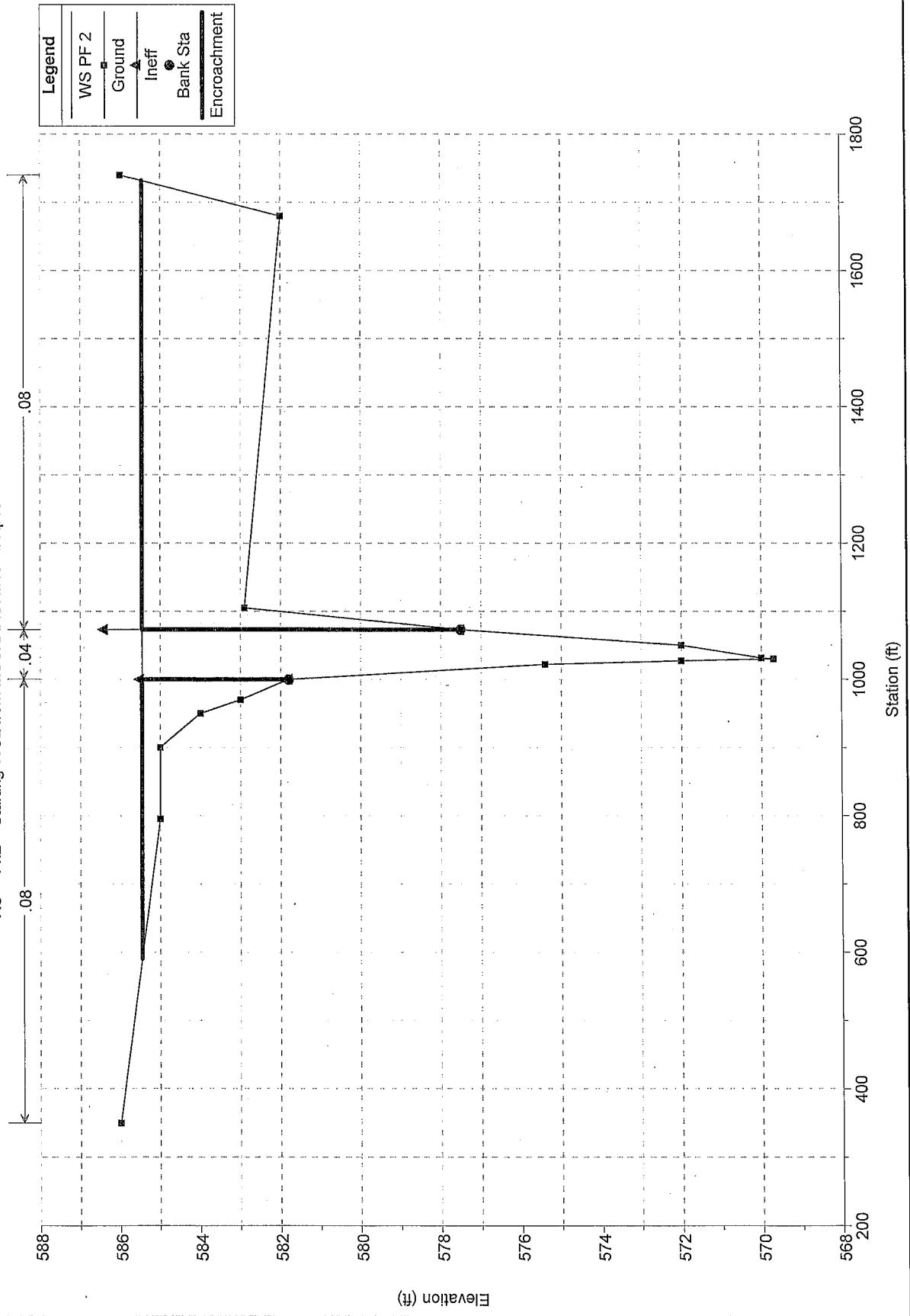
Floodway width summary: MILL CREEK
Profile No. 2

Section Number	Elevation Increase	Top Width	Left Encroach Station	Left Sta Distance From Center	Center Station	Right Sta Distance From Center	Right Encroach Station
14.200	1.00	73.00	1000.00	36.50	1036.50	36.50	1073.00
14.300	.78	150.00	970.00	66.50	1036.50	83.50	1120.00
14.400	.61	600.00	1800.00	233.50	2033.50	366.50	2400.00
15.000	.61	360.00	770.00	255.00	1025.00	105.00	1130.00
15.500	.62	412.00	1515.00	353.00	1868.00	59.00	1927.00
16.000	.60	520.00	1090.00	486.50	1576.50	33.50	1610.00
17.100	.60	562.00	2260.00	525.00	2785.00	37.00	2822.00
17.200	.57	415.57	2265.22	365.58	2630.80	49.99	2680.79
17.300	.57	420.36	2260.44	370.36	2630.80	49.99	2680.79
17.400	.63	1040.00	2050.00	997.00	3047.00	43.00	3090.00

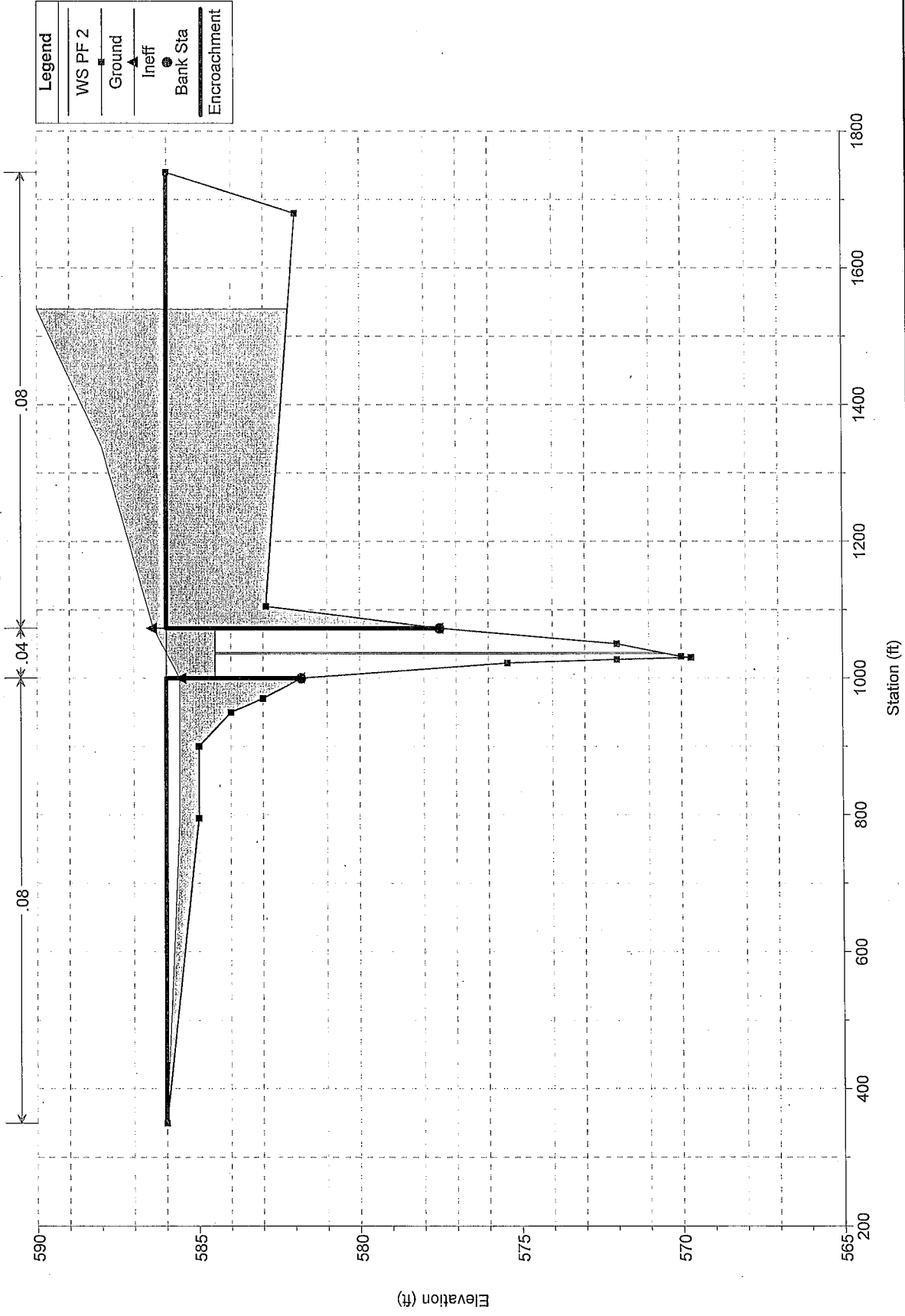
FLOODWAY DATA, MILL CREEK
PROFILE NO. 2

STATION	WIDTH	FLOODWAY SECTION AREA	MEAN VELOCITY	WATER SURFACE ELEVATION WITH FLOODWAY	WITHOUT FLOODWAY	DIFFERENCE
14.200	73.	791.	5.6	585.5	584.5	1.0
14.300	103.	920.	4.9	585.8	585.0	.8
14.400	600.	3550.	1.3	586.3	585.7	.6
15.000	360.	1873.	2.4	586.6	586.0	.6
15.500	385.	2702.	1.7	586.9	586.3	.6
16.000	520.	4637.	1.0	587.0	586.4	.6
17.100	562.	4454.	1.0	587.0	586.4	.6
17.200	410.	2417.	1.8	587.0	586.4	.6
17.300	415.	2446.	1.8	587.0	586.4	.6
17.400	1035.	4087.	1.1	587.2	586.6	.6

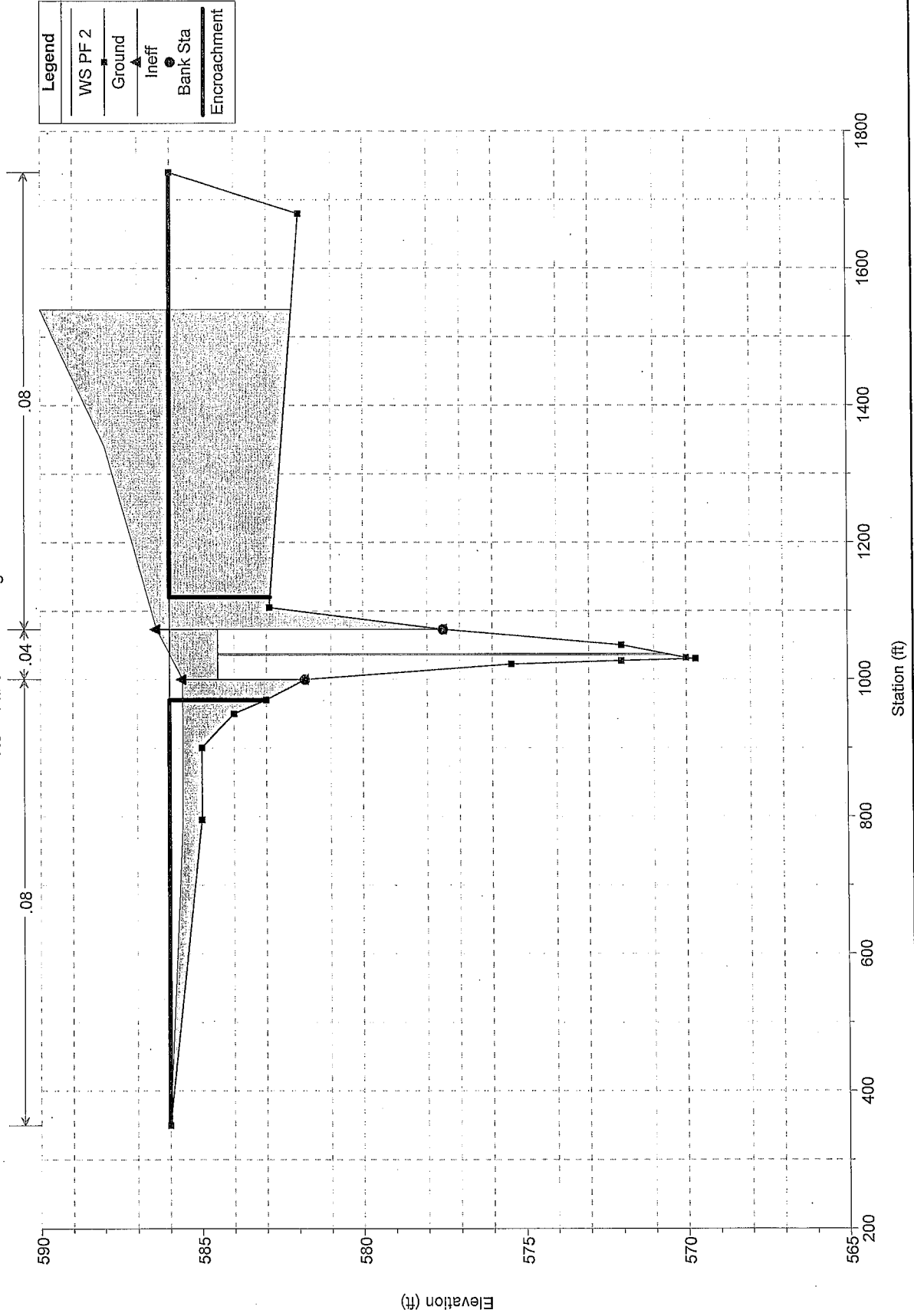
Mill Creek Floodway Duplicate Effective
RS = 14.2 Starting WSL from MCSHARON.HC2 output



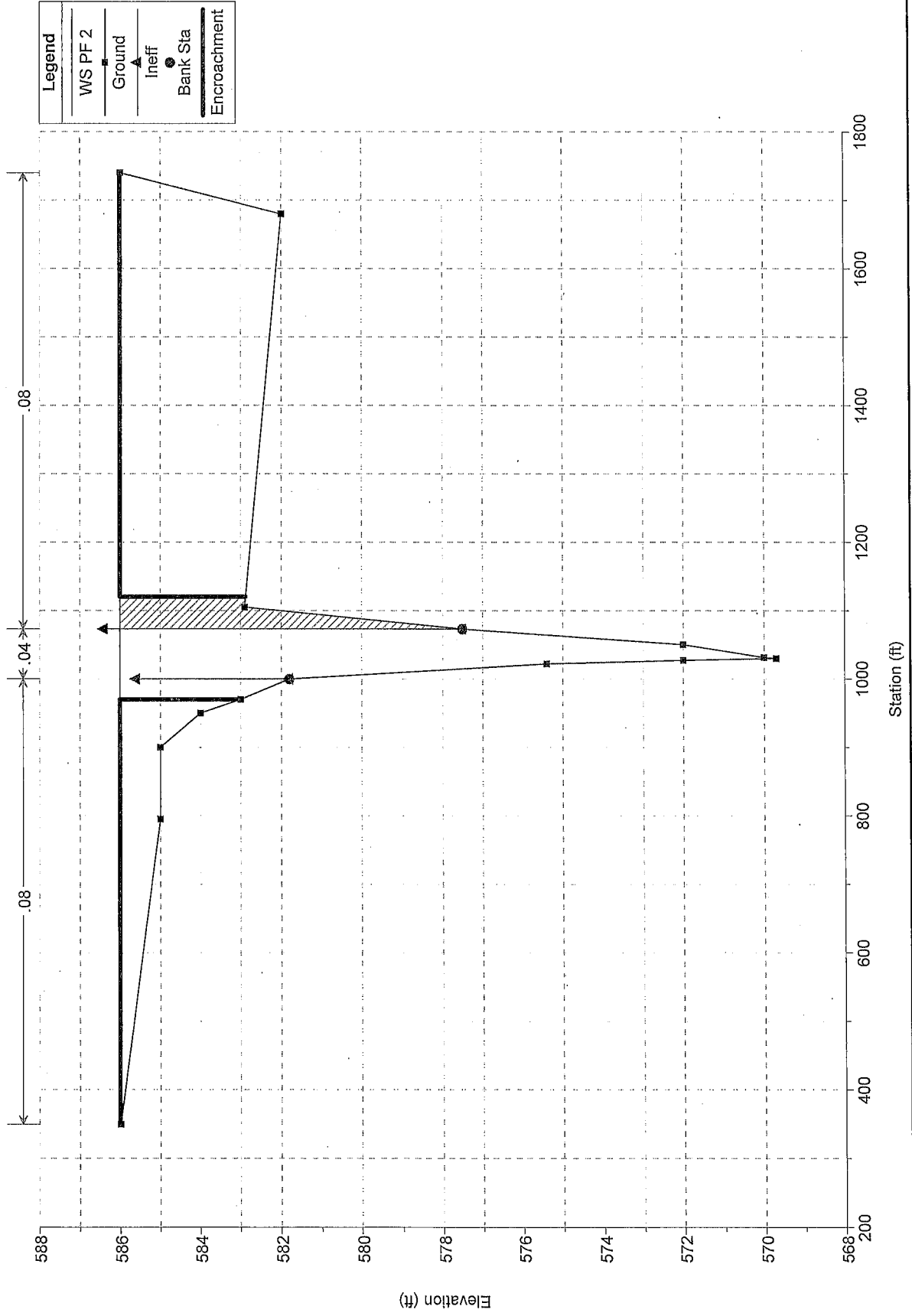
Mill Creek Floodway Duplicate Effective
RS = 14.25 BR Bridge #1



Mill Creek Floodway Duplicate Effective RS = 14.25 BR Bridge #1

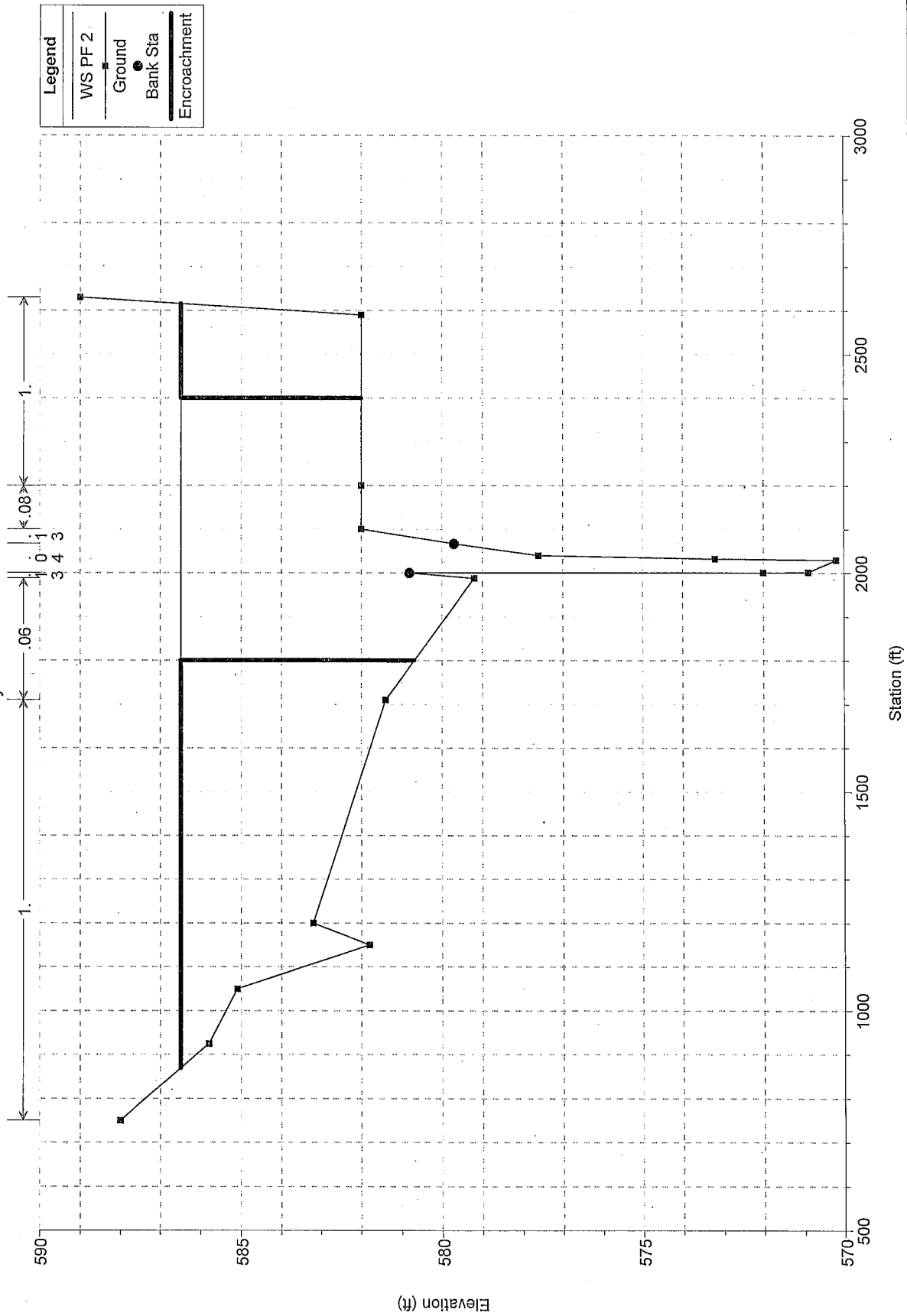


Mill Creek Floodway Duplicate Effective
RS = 14.3 This is a REPEATED section.



Mill Creek Floodway Duplicate Effective

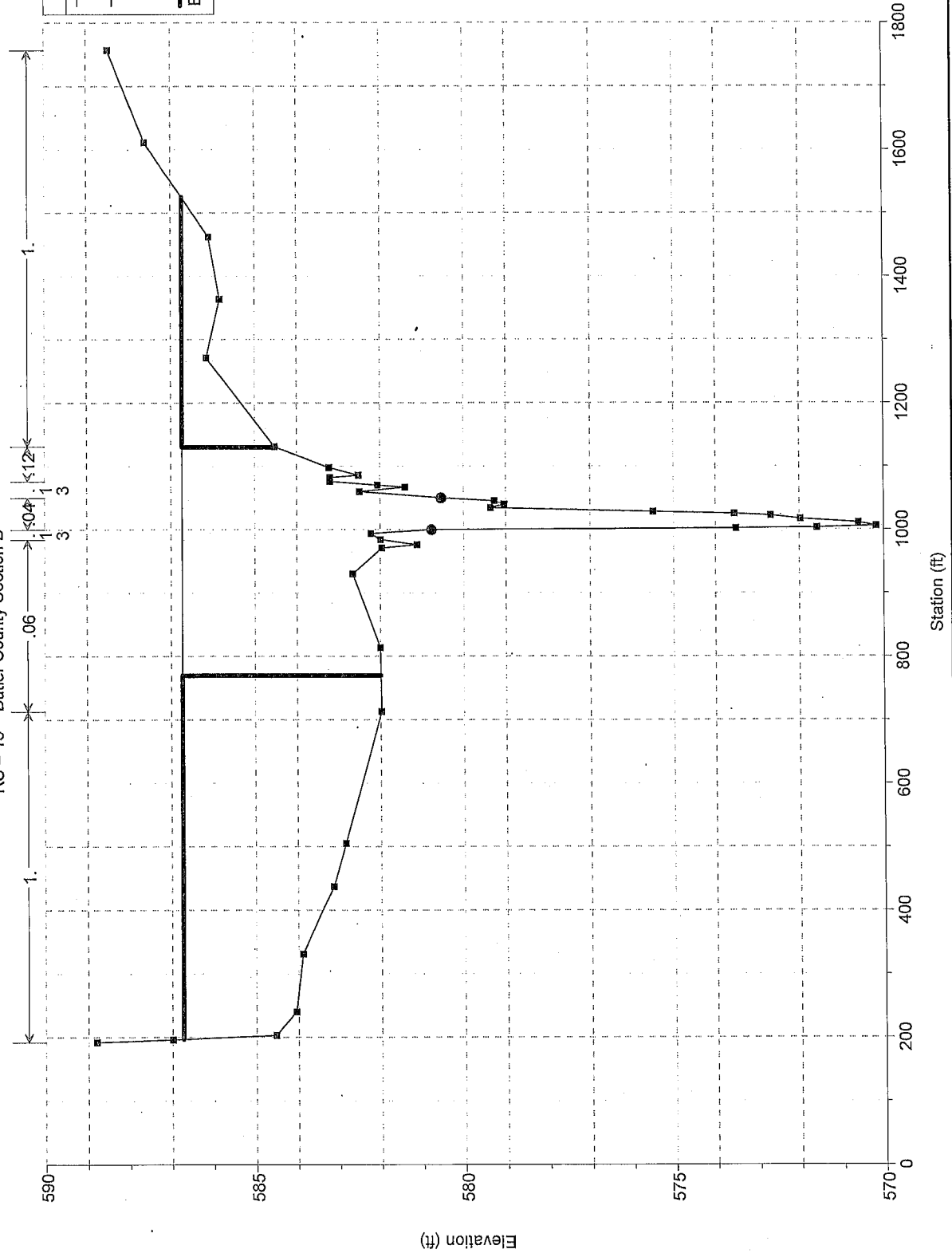
RS = 14.4 Butler County Section A



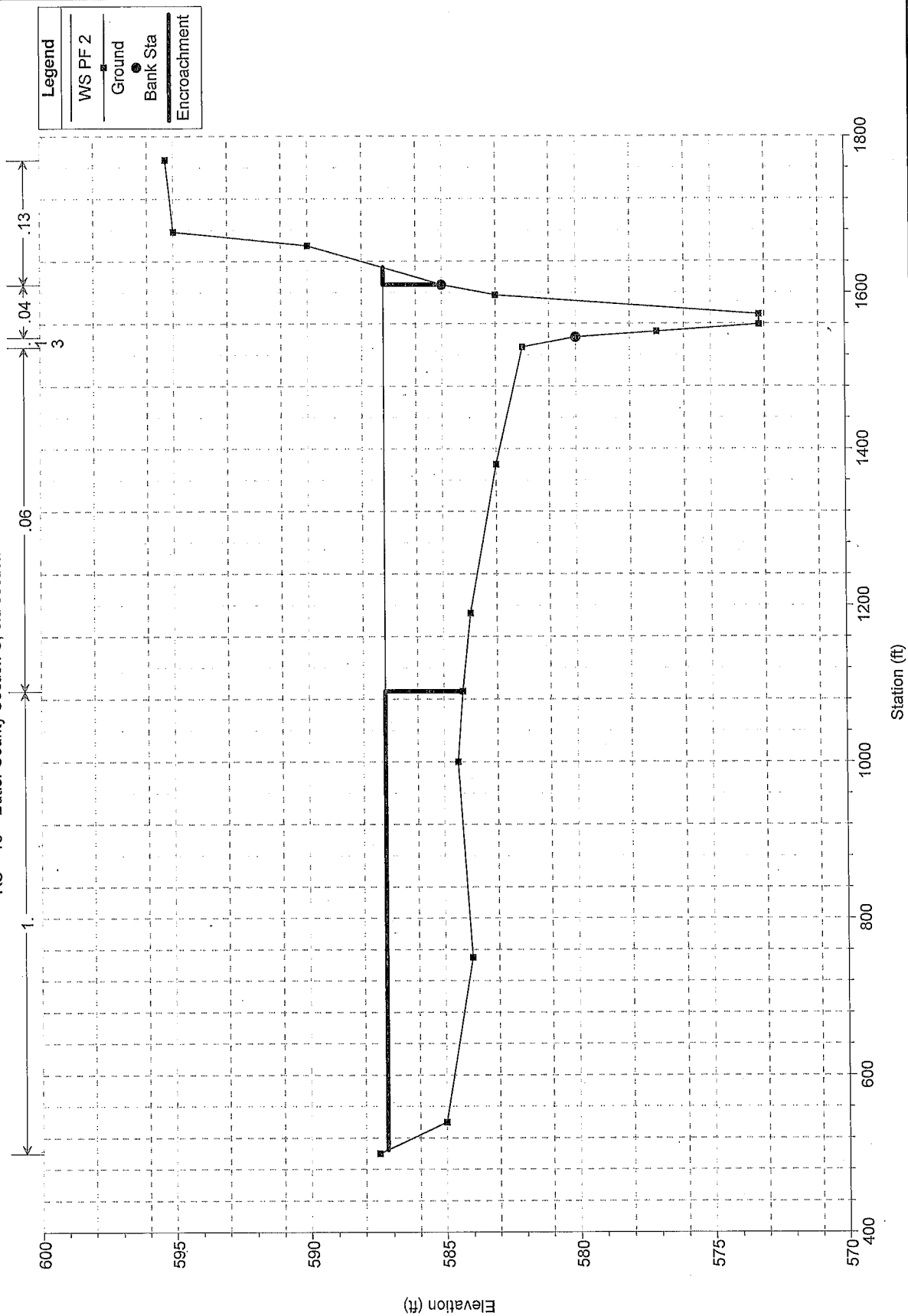
Mill Creek Floodway Duplicate Effective

RS = 15 Butler County Section B

Legend
WS PF 2
Ground
Bank Sta
Encroachment

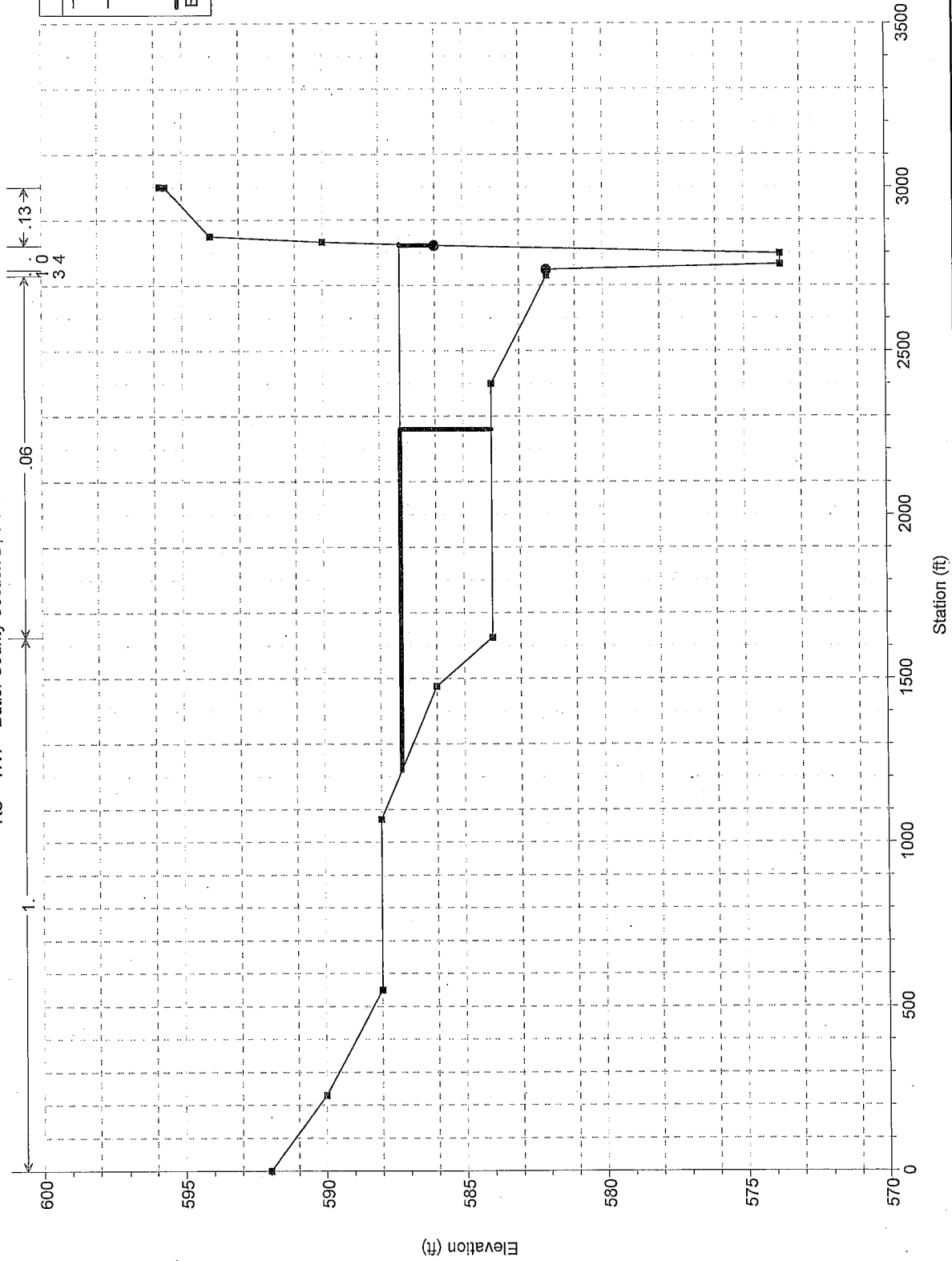


RS = 16 Butler County Section C, old section D

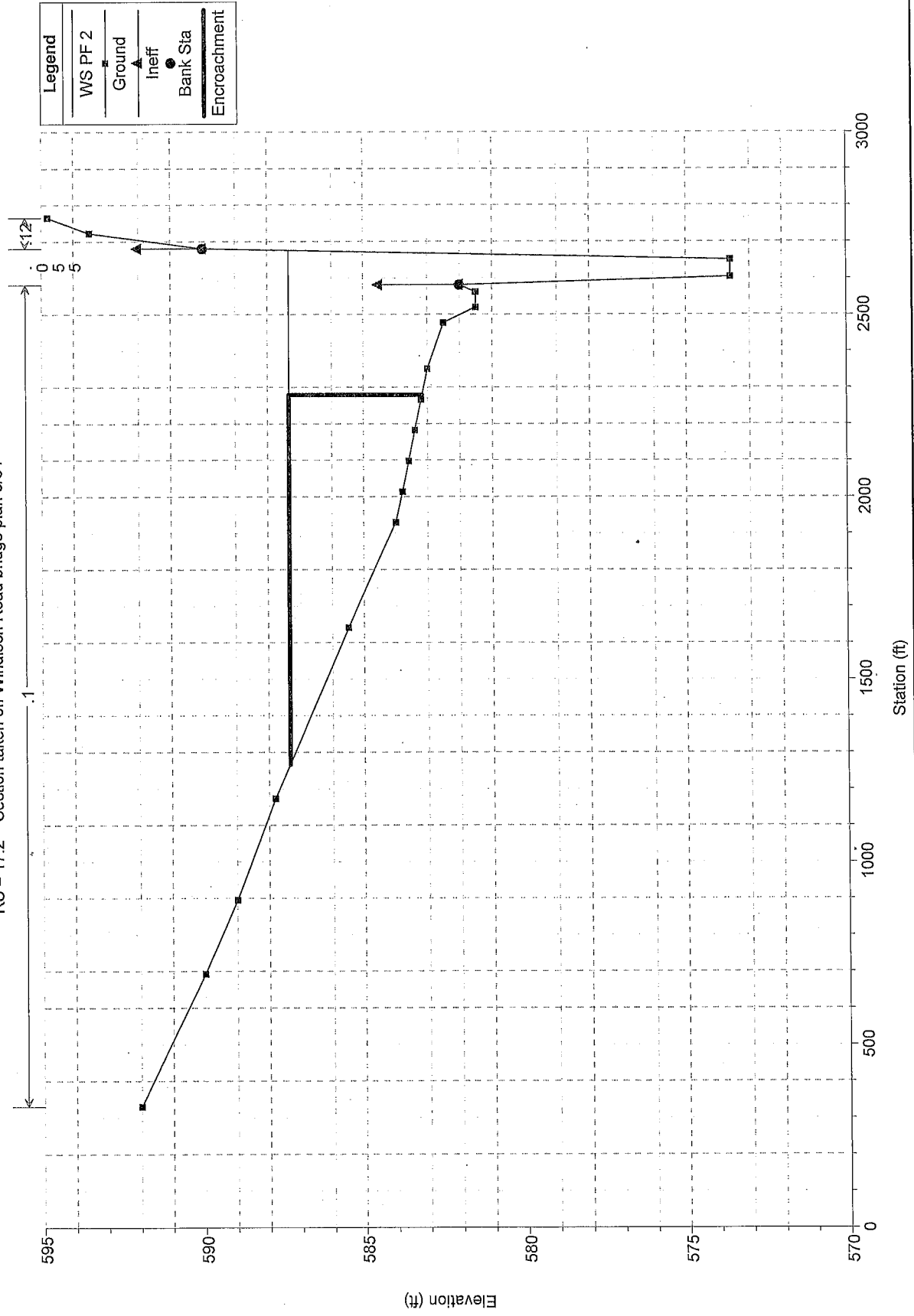


Mill Creek Floodway Duplicate Effective
 RS = 17.1 Butler County Section D, old section E

Legend
WS PF 2
Ground
Bank Sta
Encroachment

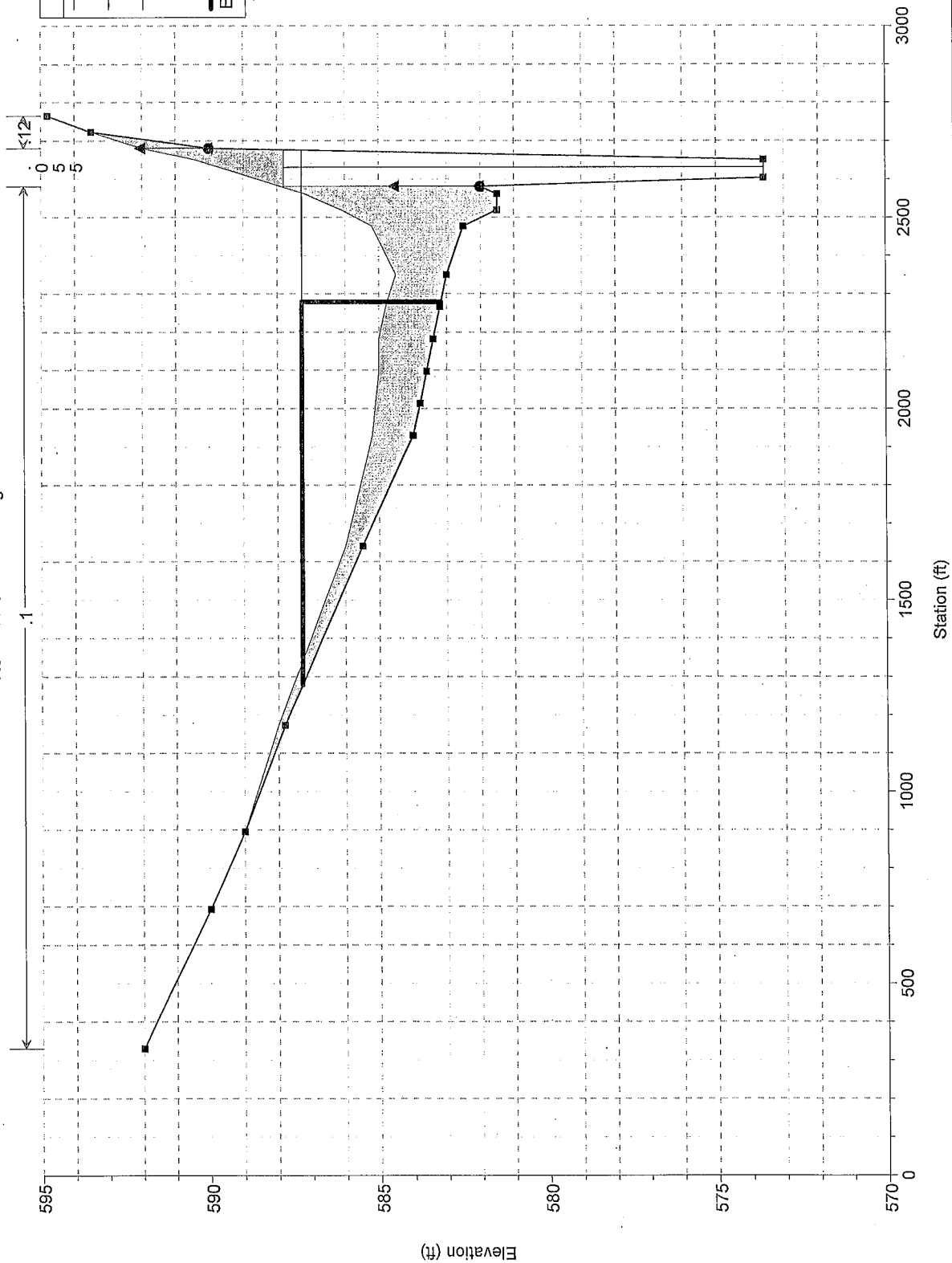


Mill Creek Floodway Duplicate Effective RS = 17.2 Section taken off Windisch Road bridge plan 3/84

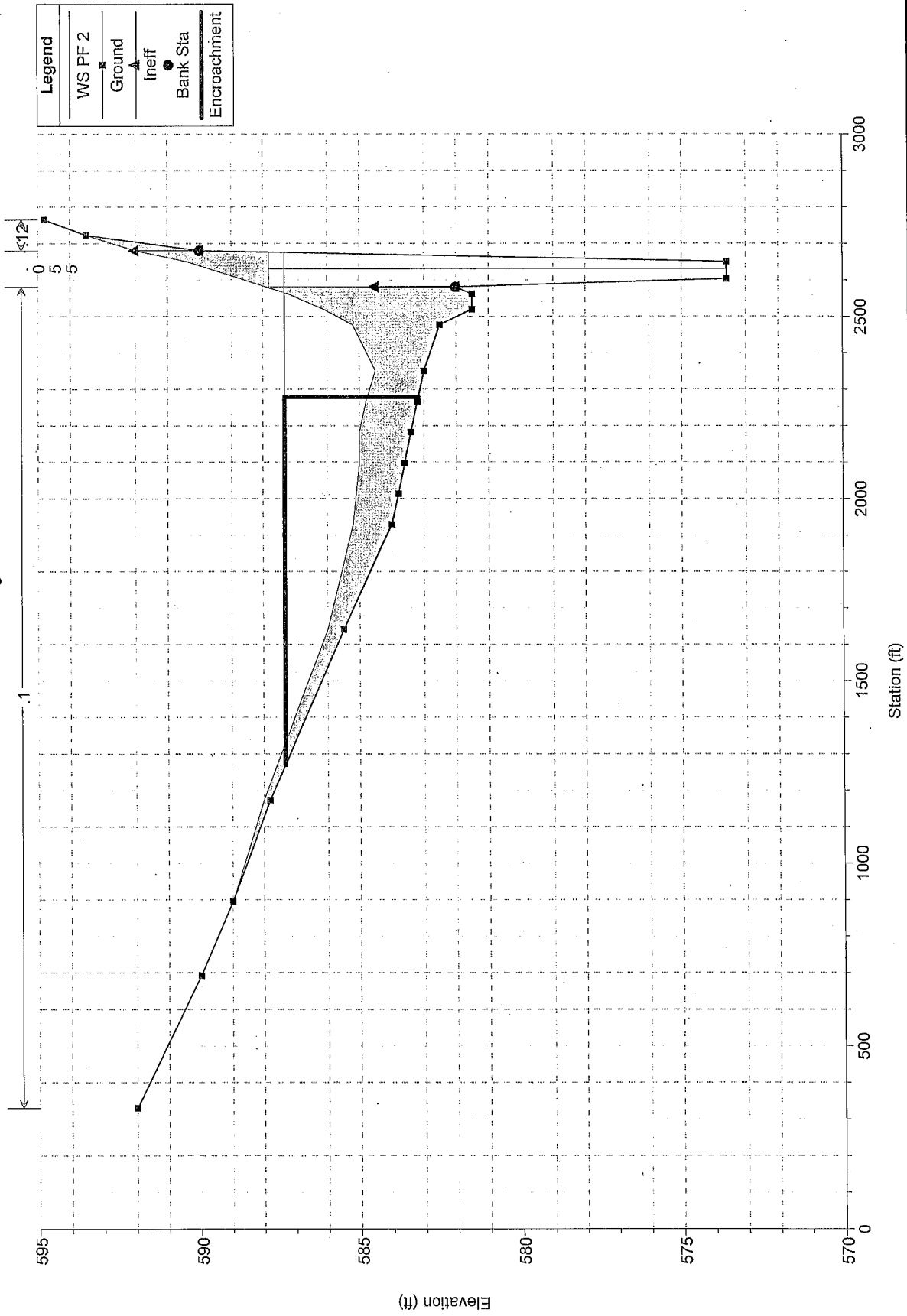


Mill Creek Floodway Duplicate Effective RS = 17.25 BR Bridge #2

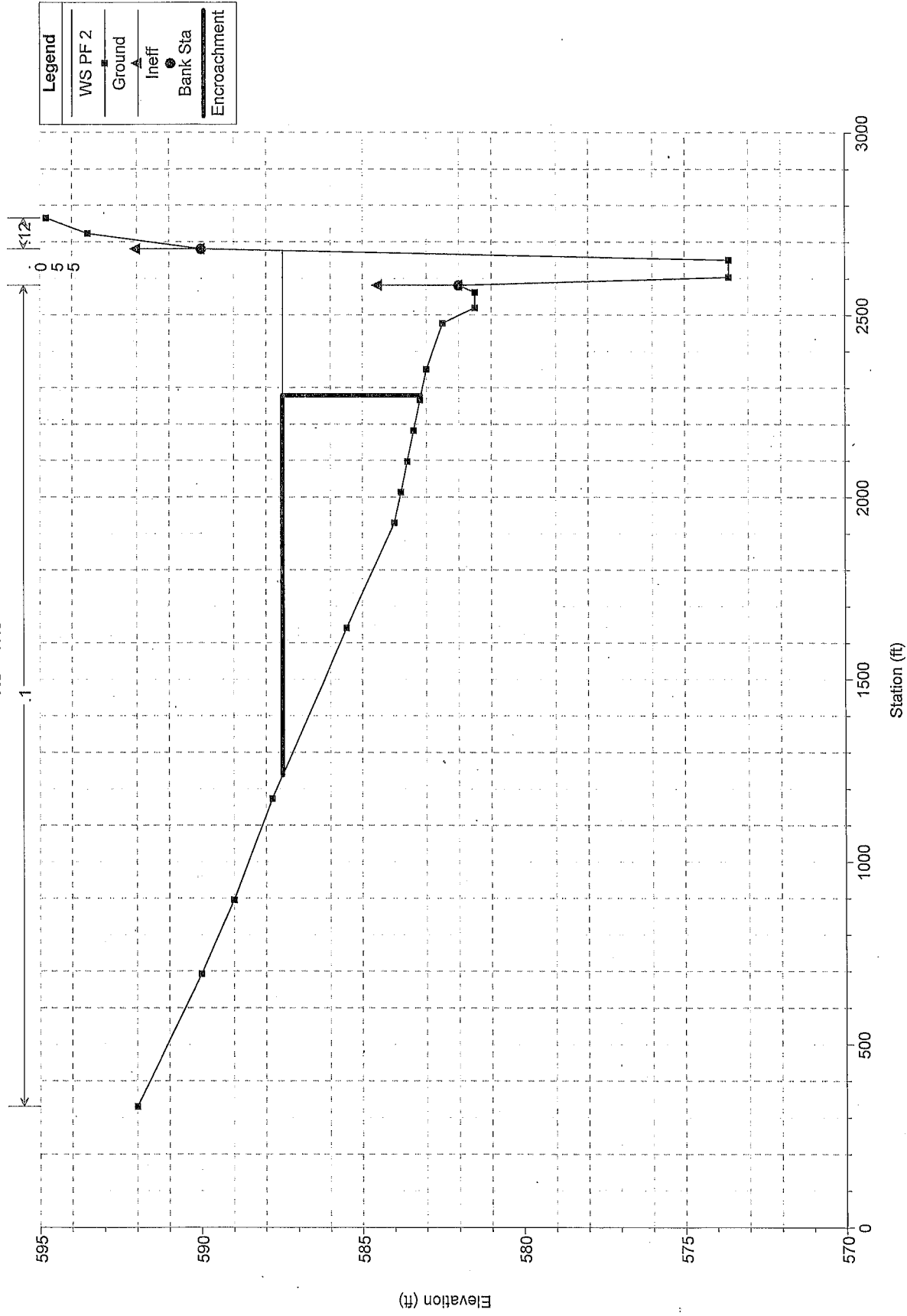
Legend	
WS PF 2	Ground
Ineff	Bank Sta
Encroachment	



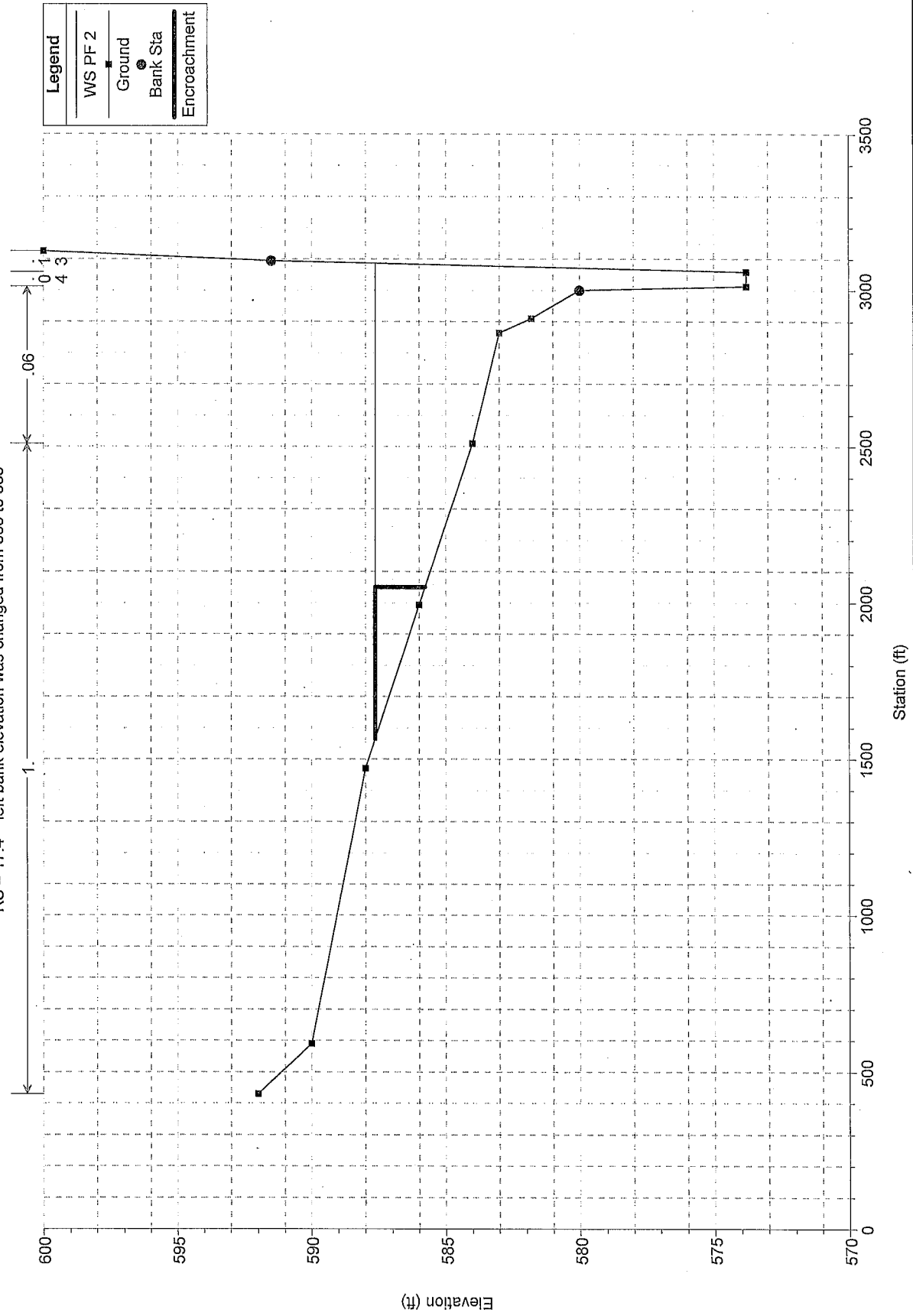
Mill Creek Floodway Duplicate Effective RS = 17.25 BR Bridge #2



Mill Creek Floodway Duplicate Effective RS = 17.3

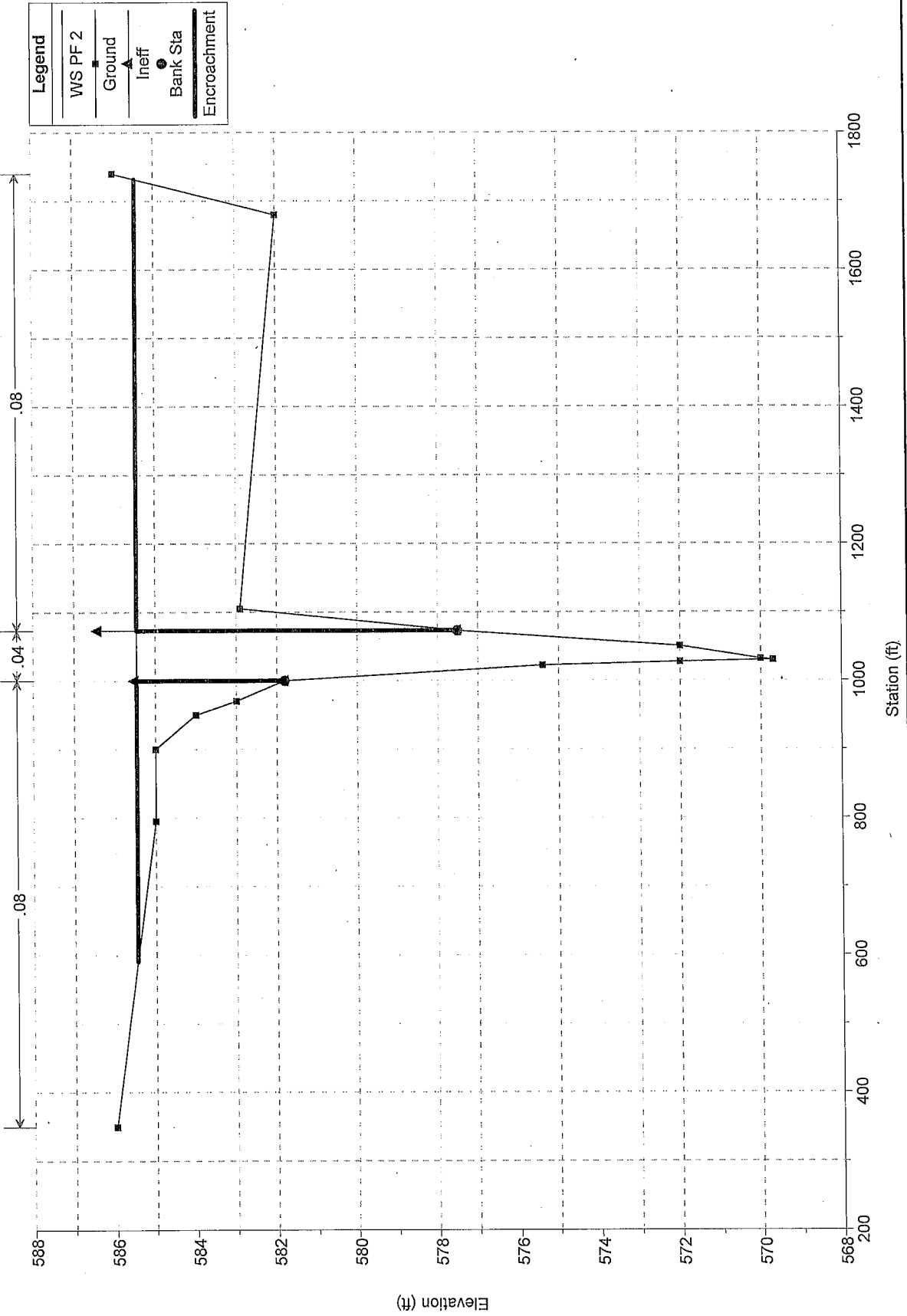


Mill Creek Floodway Duplicate Effective RS = 17.4 left bank elevation was changed from 588 to 583



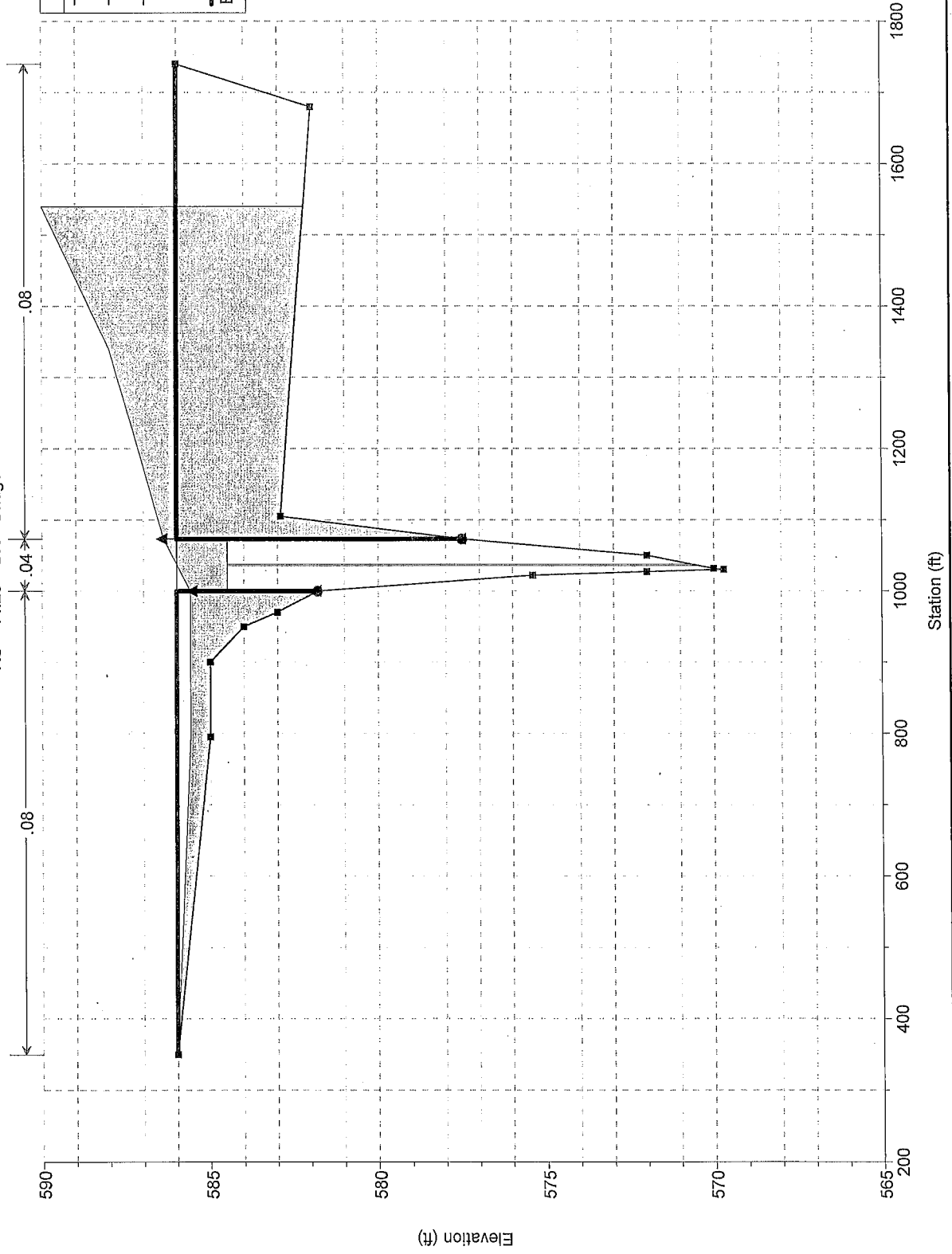
Mill Creek Floodway - Revised Existing

RS = 14.2 Starting WSL from MCSHARON.HC2 output

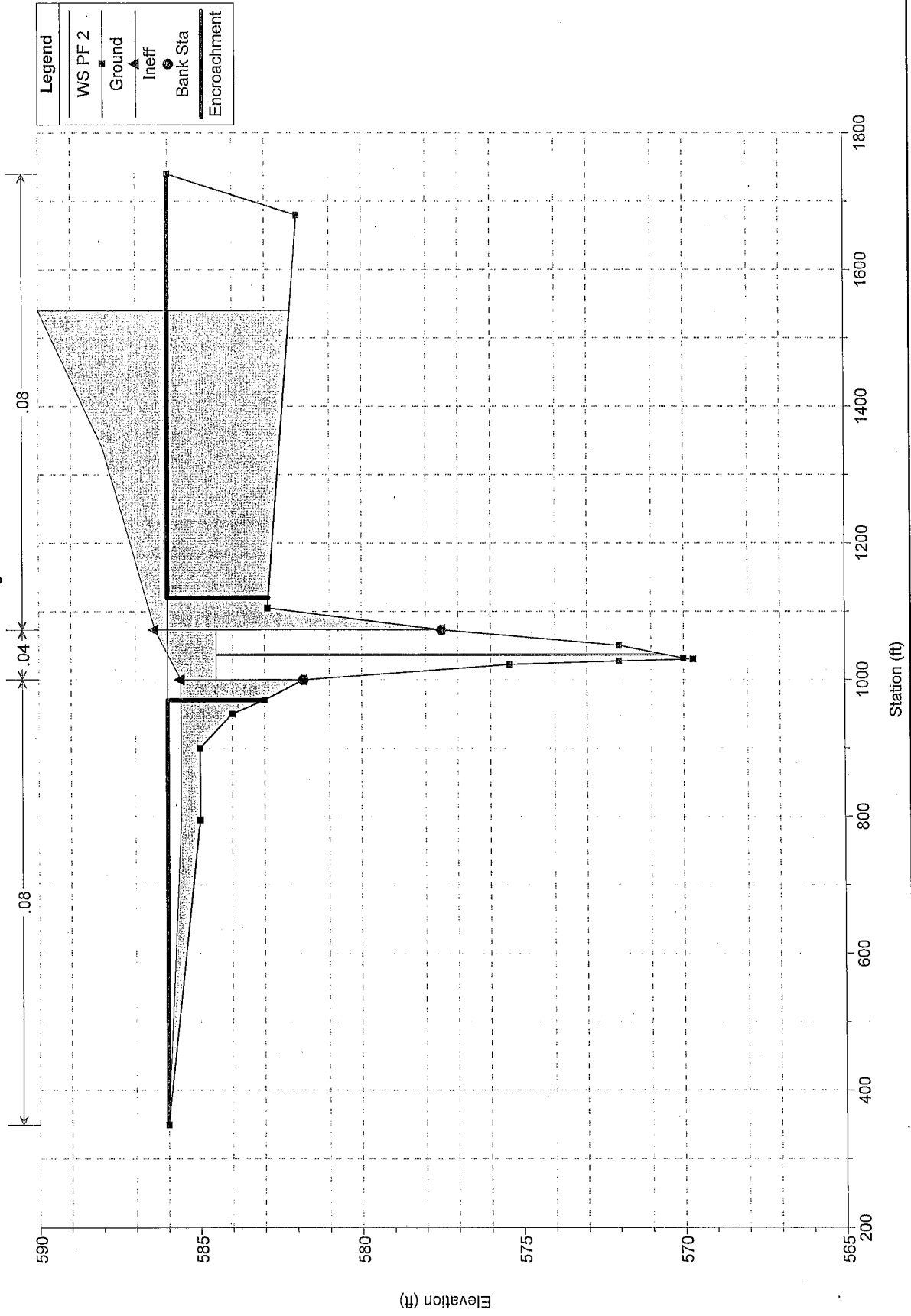


Mill Creek Floodway - Revised Existing RS = 14.25 BR Bridge #1

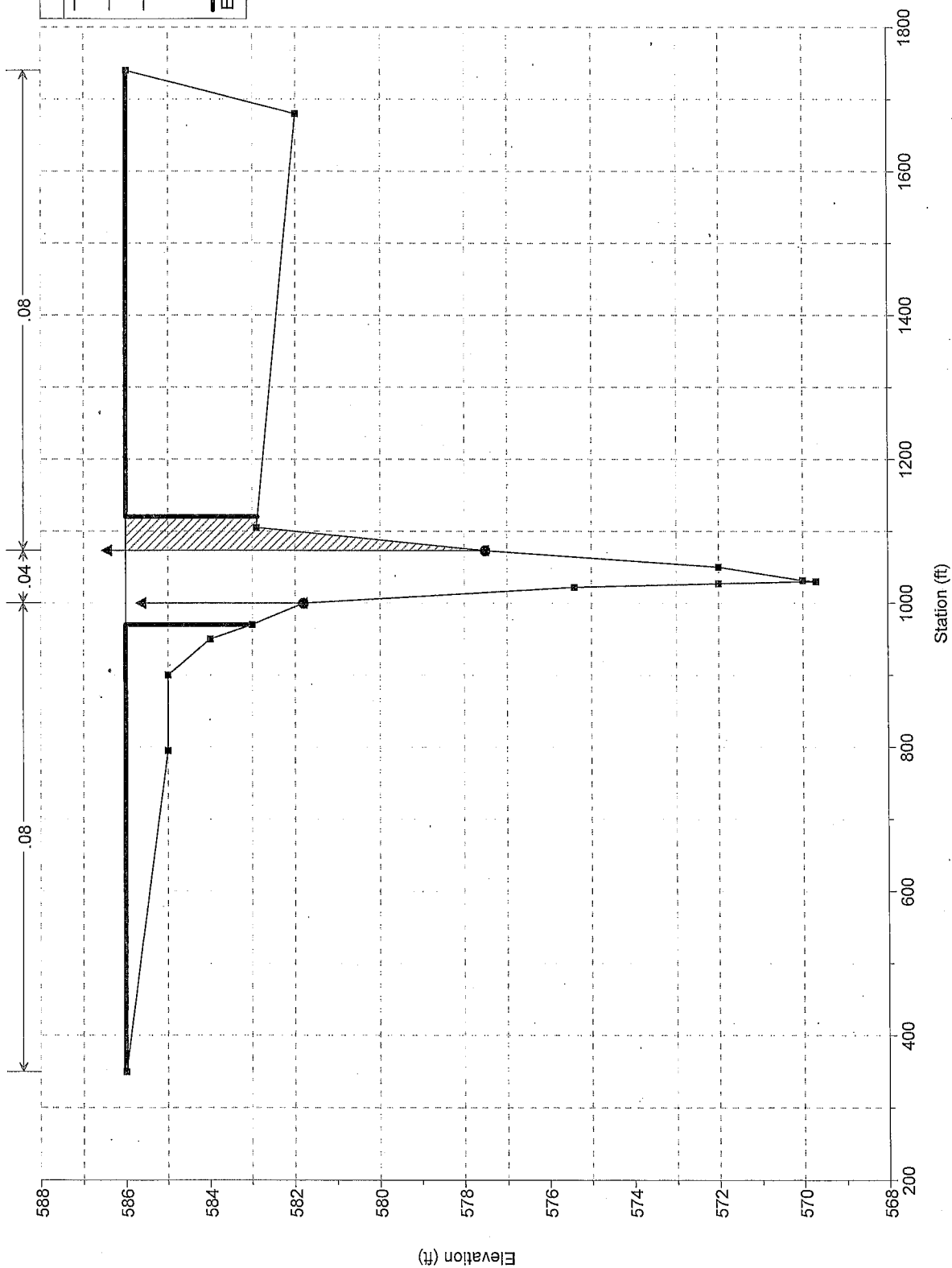
Legend
WS PF 2
Ground
Ineff
Bank Sta
Encroachment



Mill Creek Floodway - Revised Existing
 RS = 14.25 BR Bridge #1

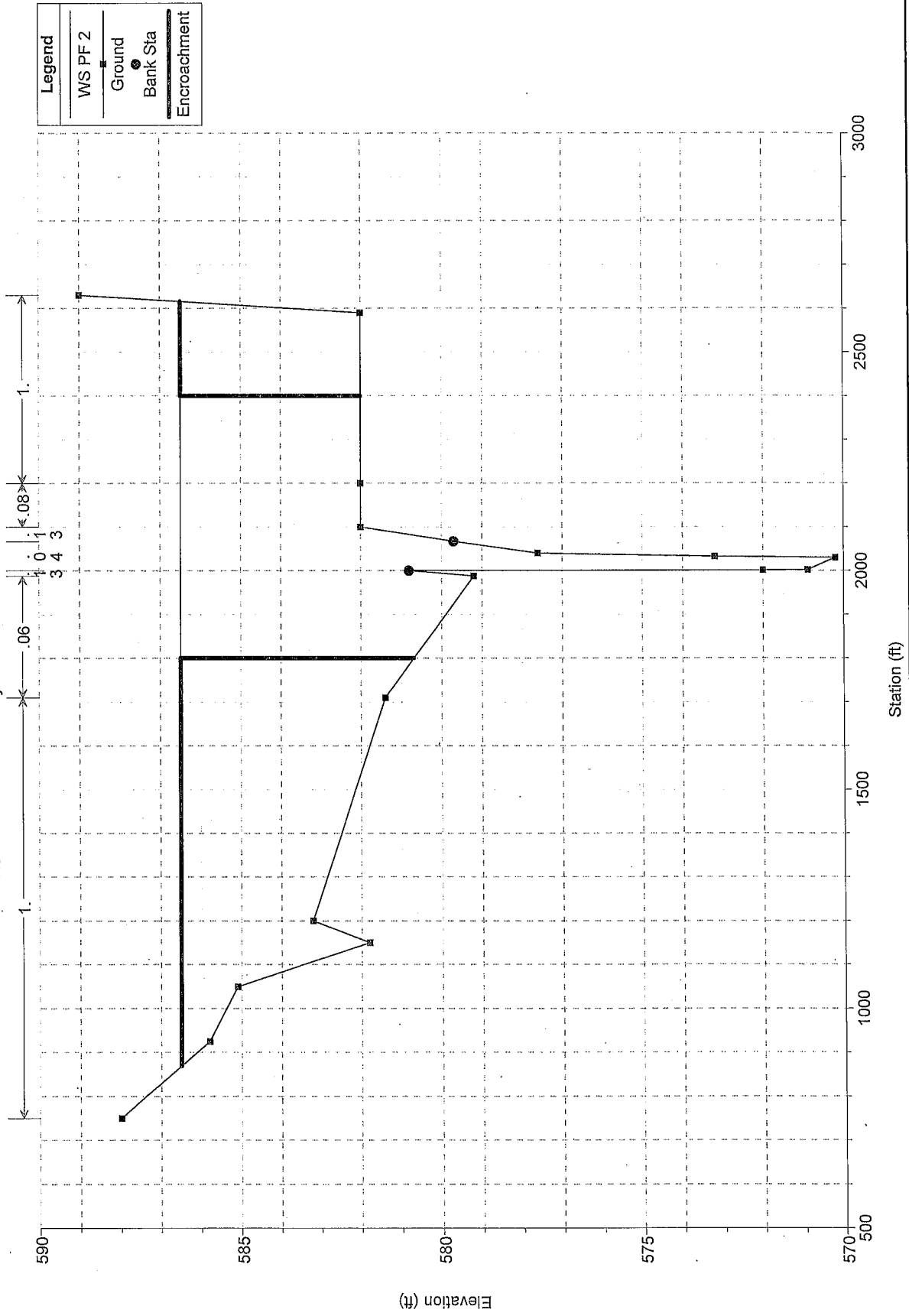


Mill Creek Floodway - Revised Existing
 RS = 14.3 This is a REPEATED section.



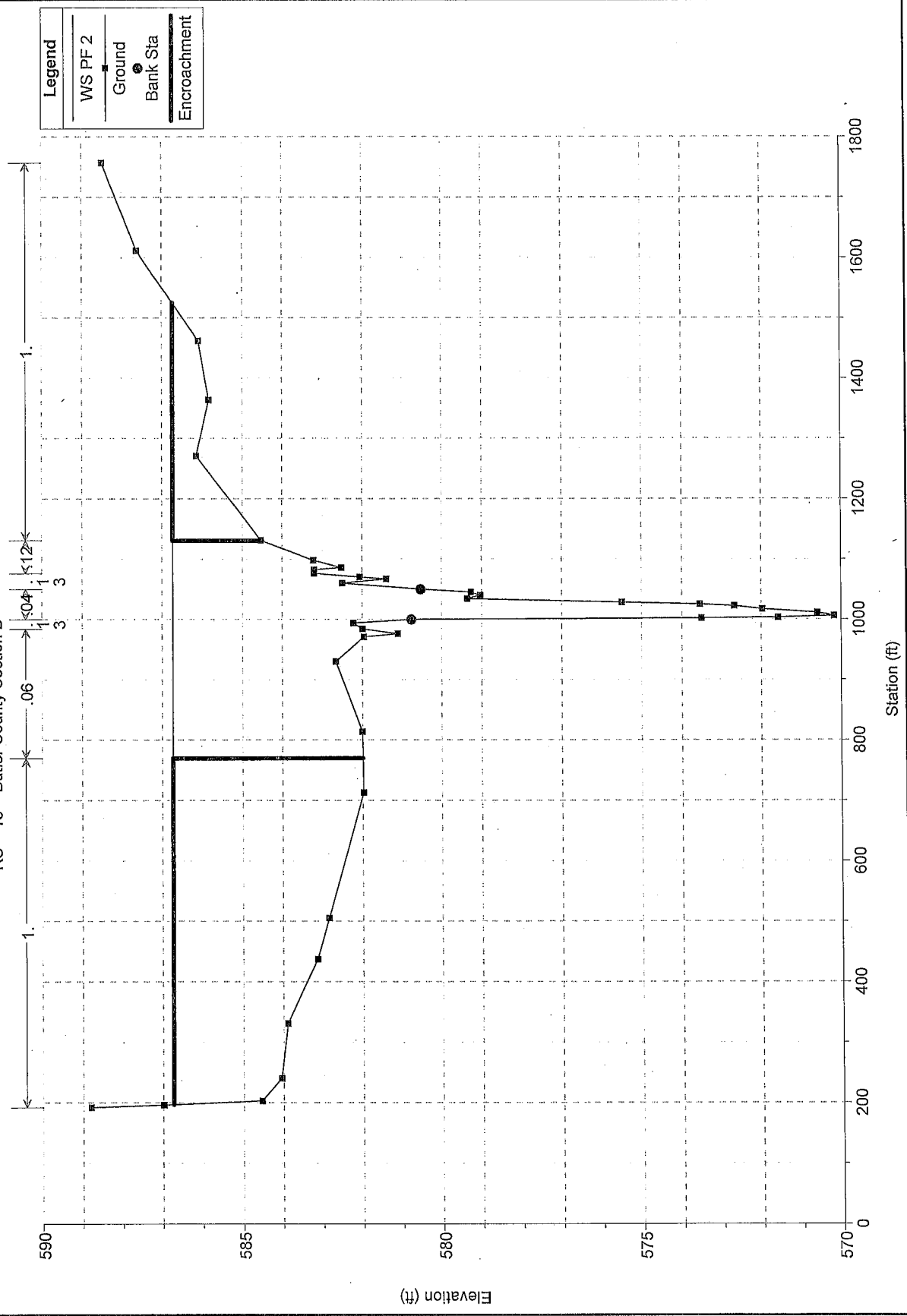
Mill Creek Floodway - Revised Existing

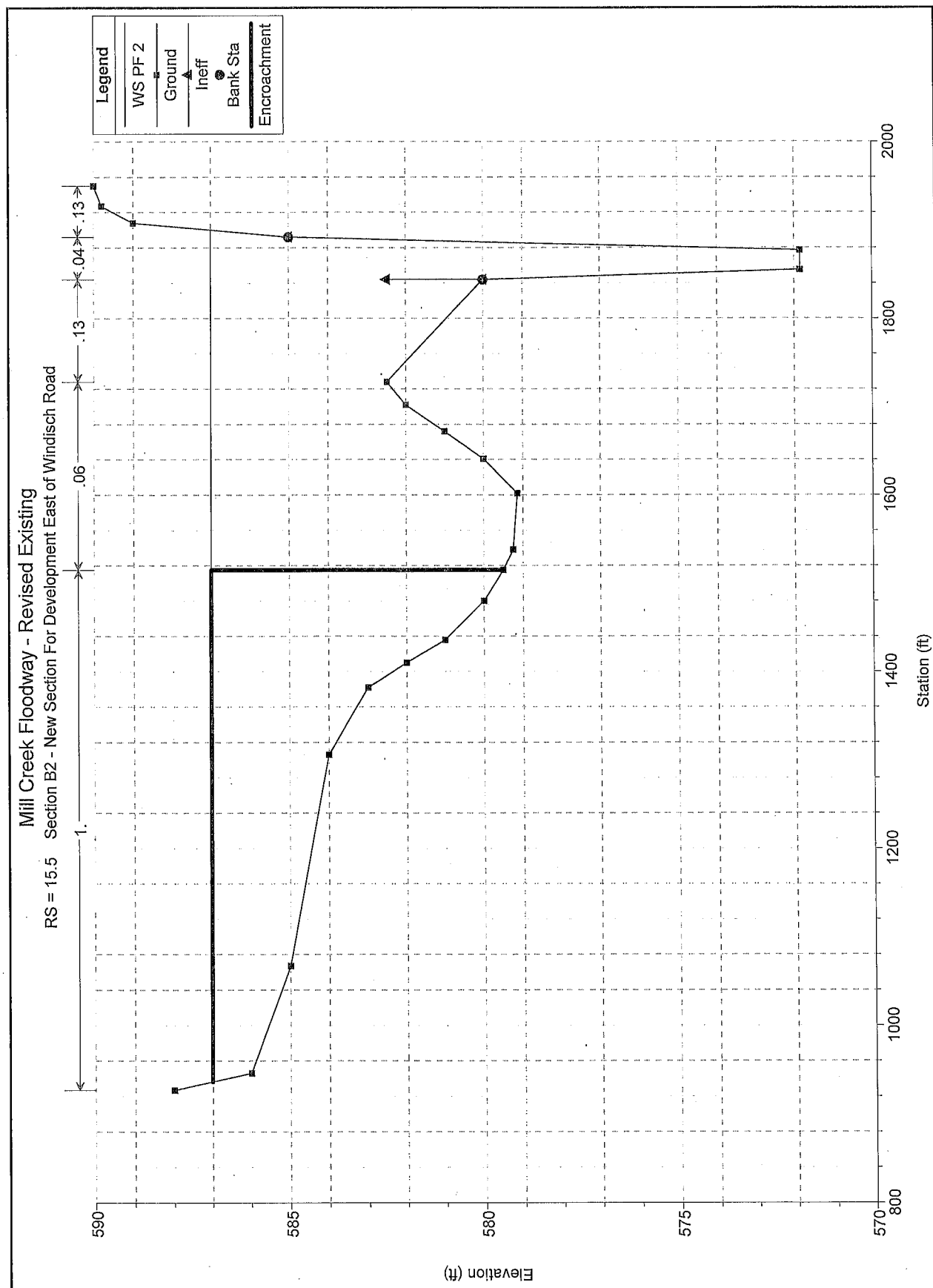
RS = 14.4 Butler County Section A



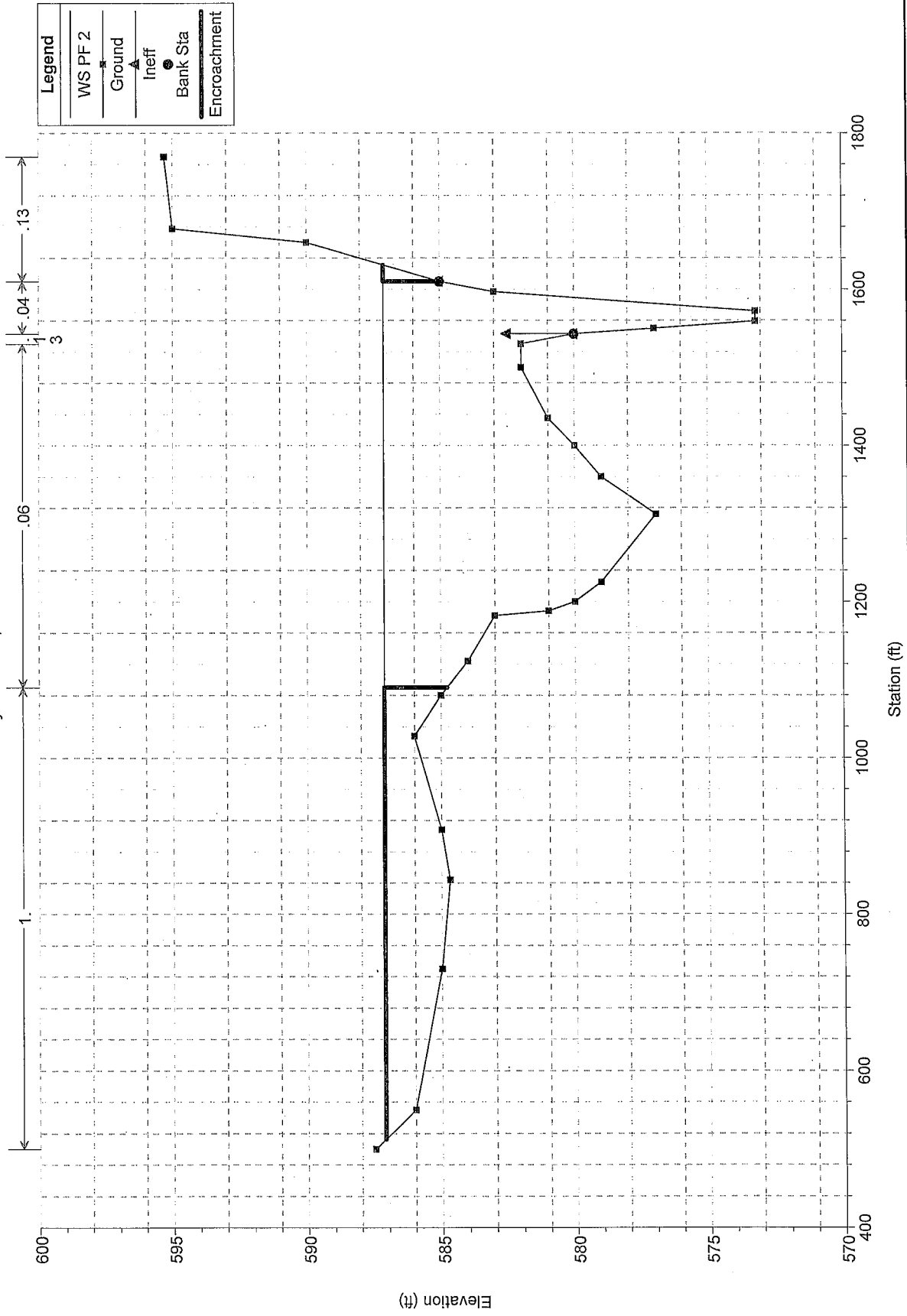
Mill Creek Floodway - Revised Existing

RS = 15 Butler County Section B

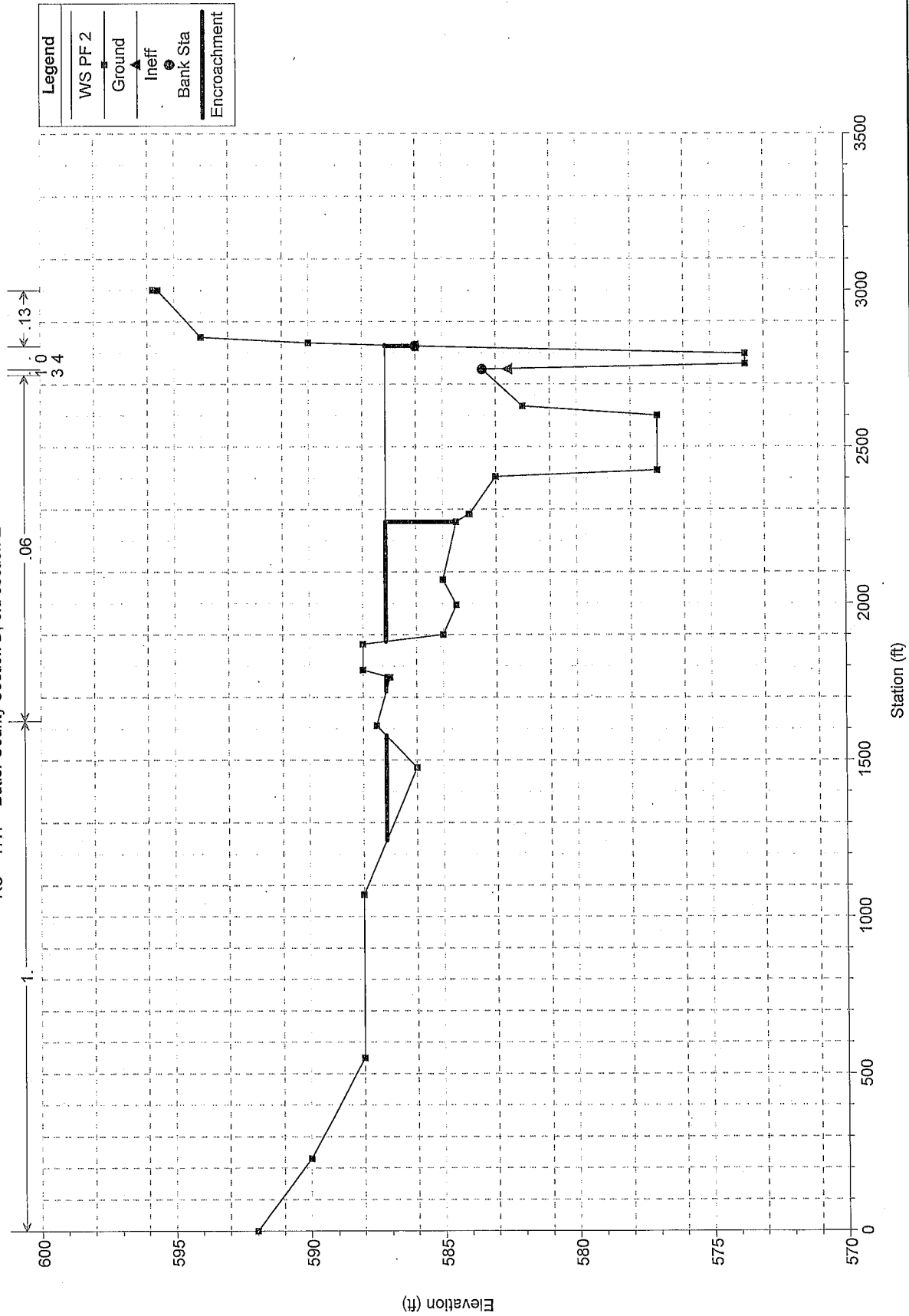




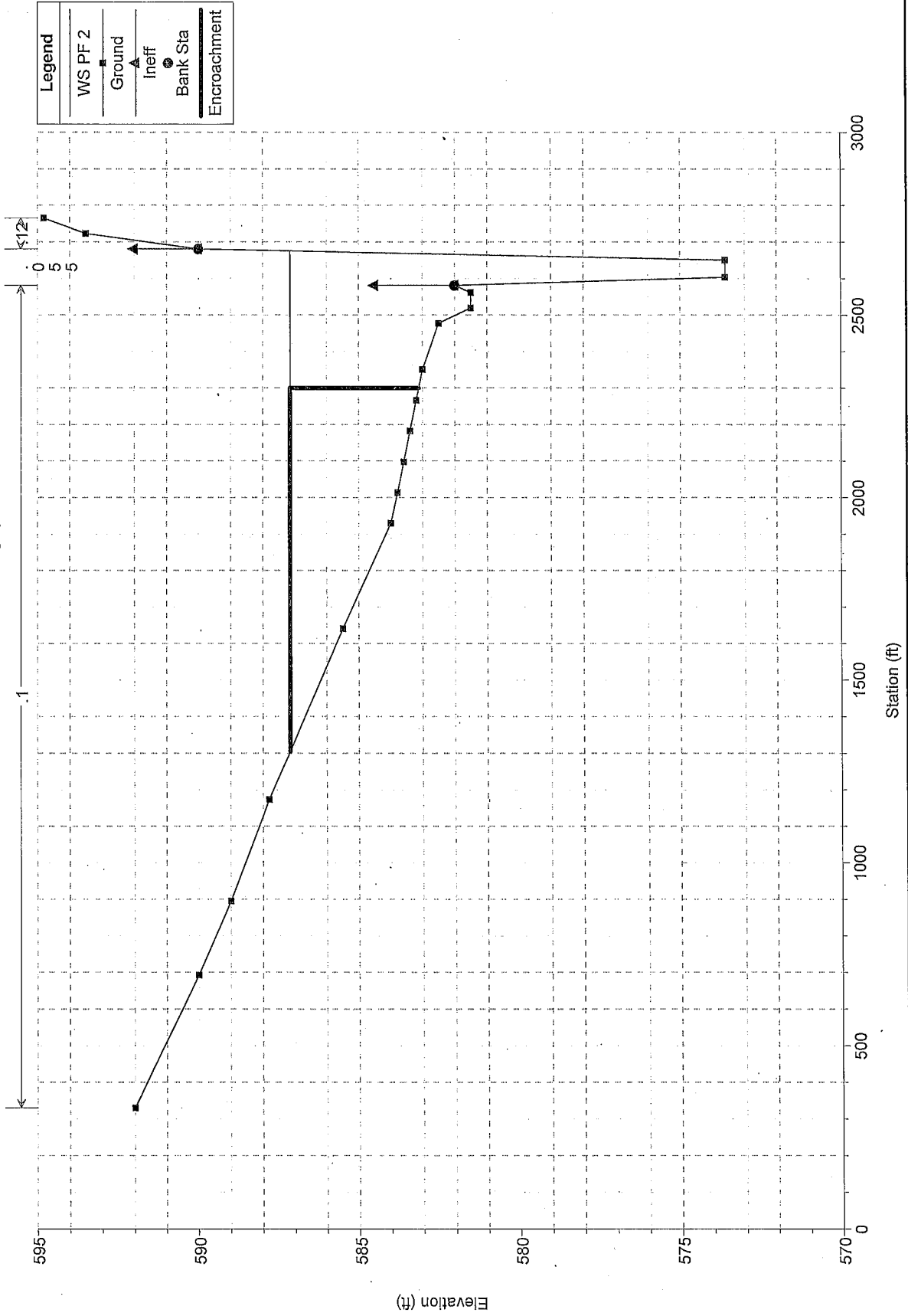
Mill Creek Floodway - Revised Existing
 RS = 16 Butler County Section C, old section D



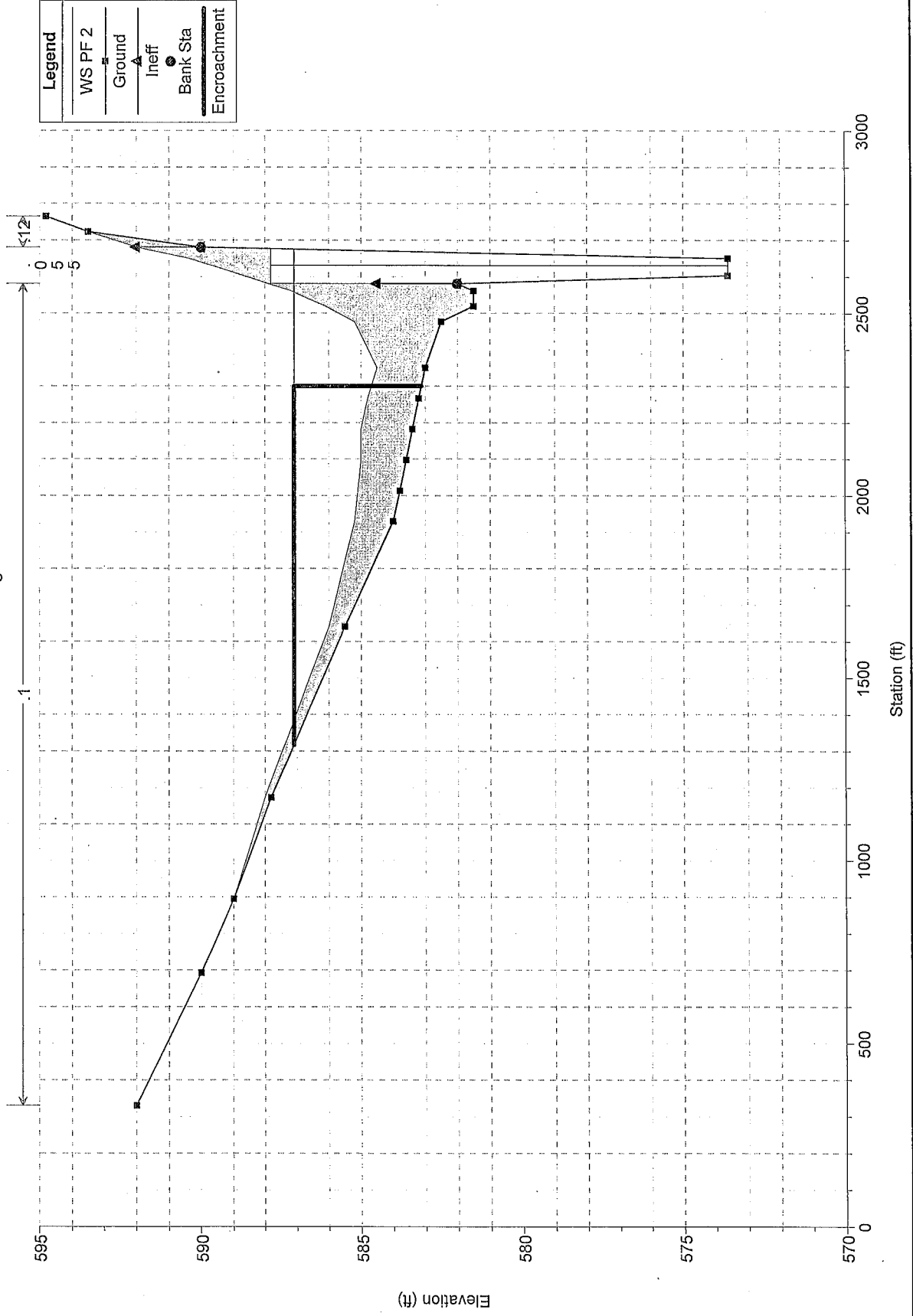
Mill Creek Floodway - Revised Existing
 RS = 17.1 Butler County Section D, old section E



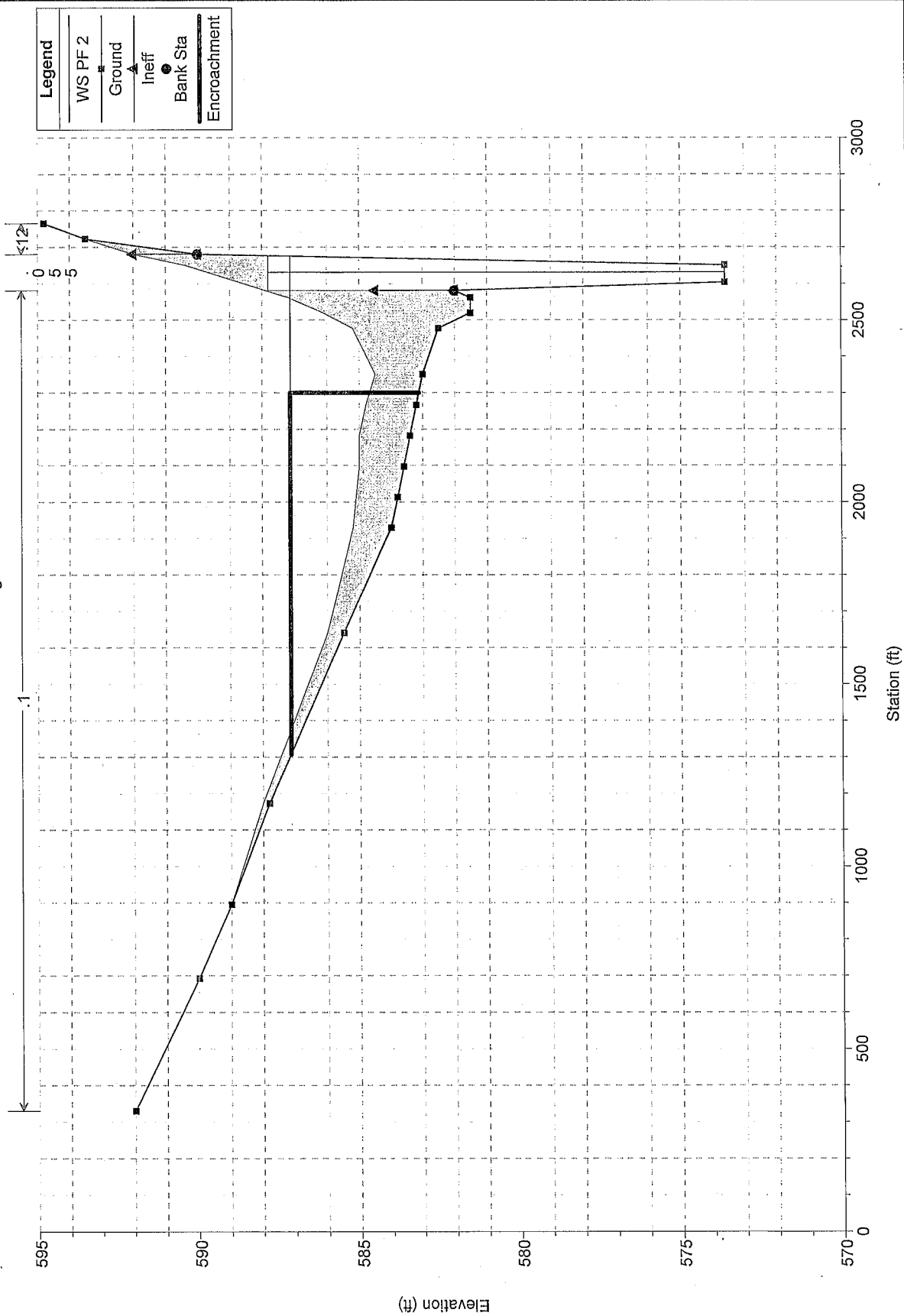
Mill Creek Floodway - Revised Existing
 RS = 17.2 Section taken off Windisch Road bridge plan 3/84



Mill Creek Floodway - Revised Existing
RS = 17.25 BR Bridge #2

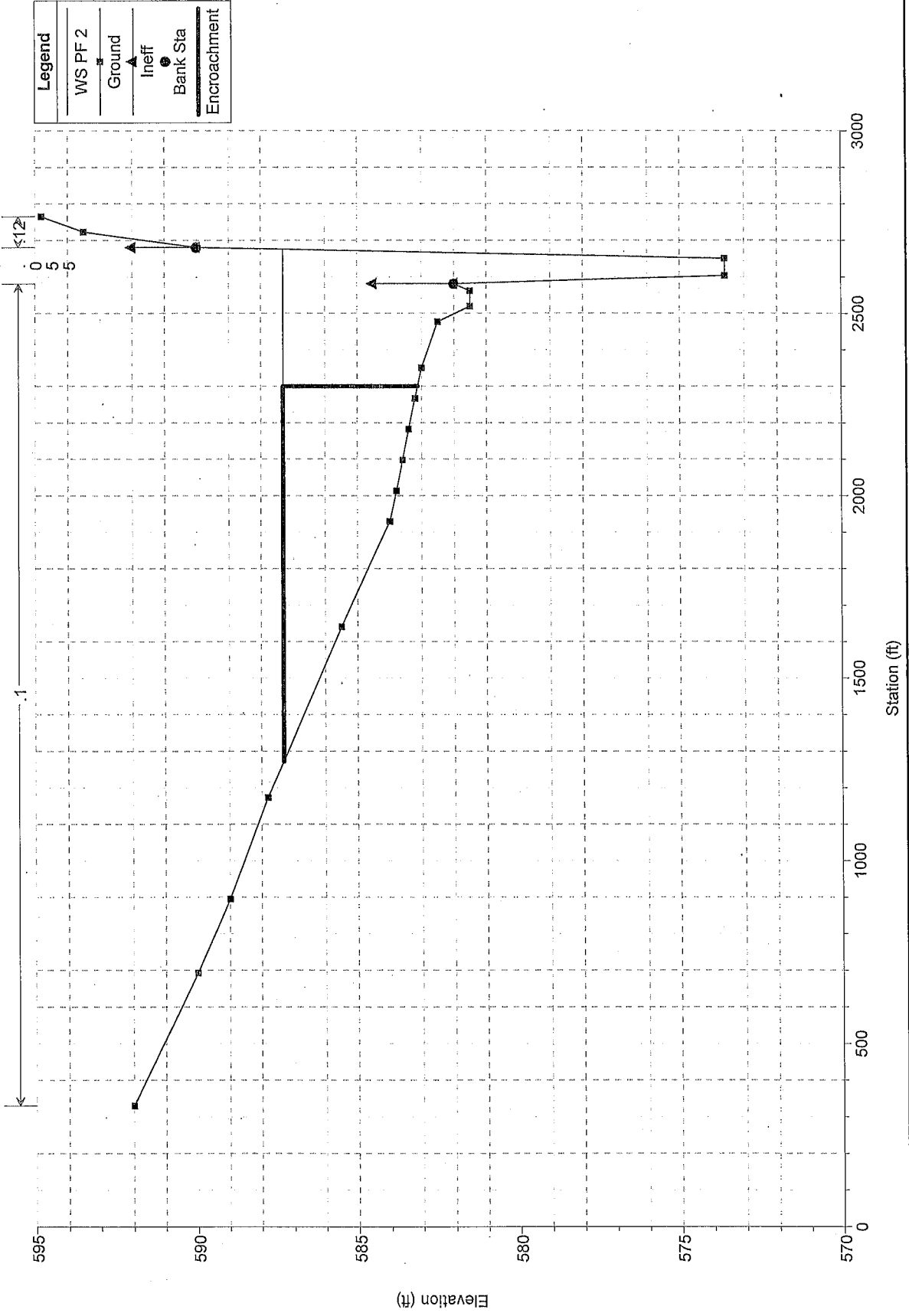


Mill Creek Floodway - Revised Existing RS = 17.25 BR Bridge #2

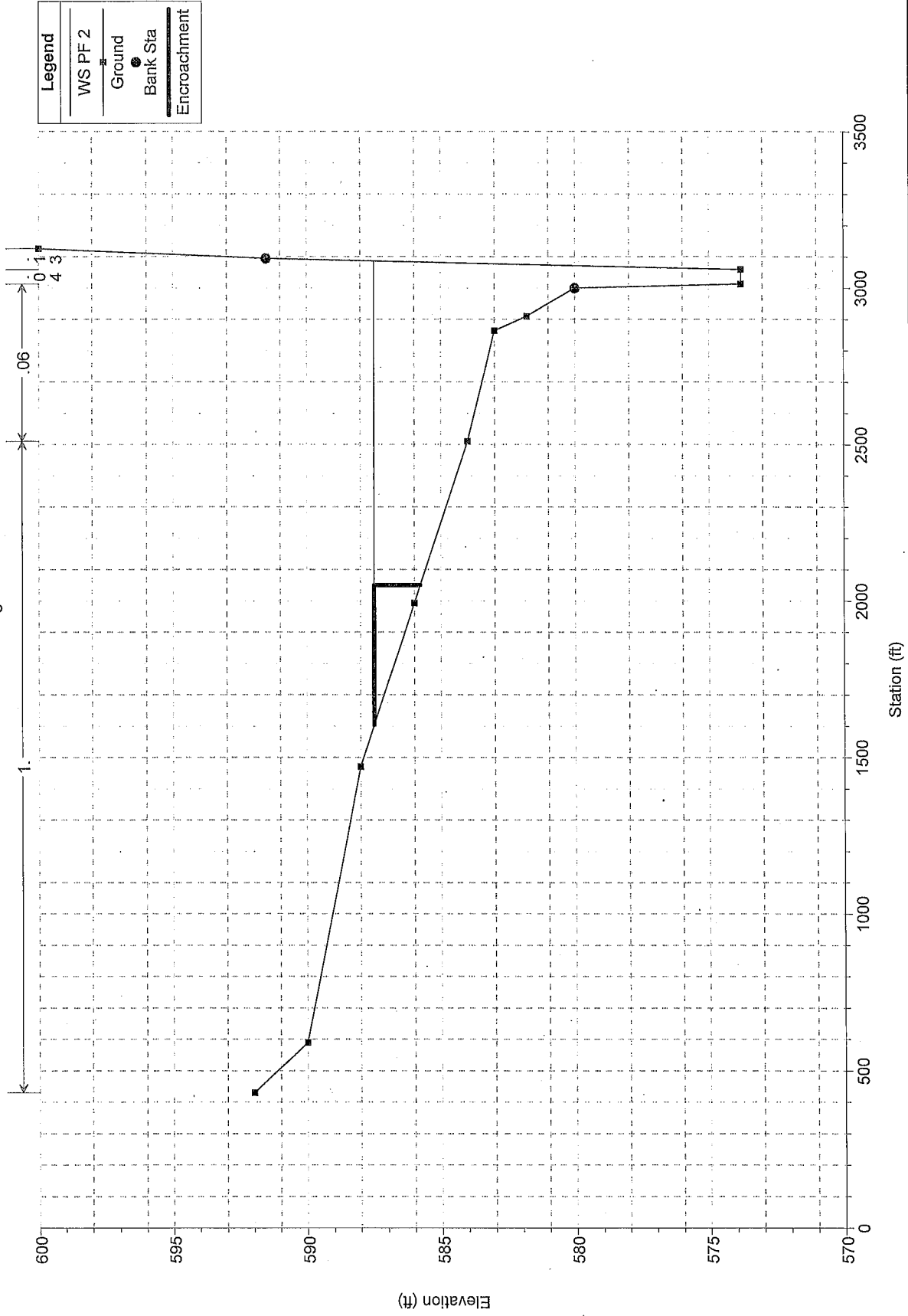


Mill Creek Floodway - Revised Existing

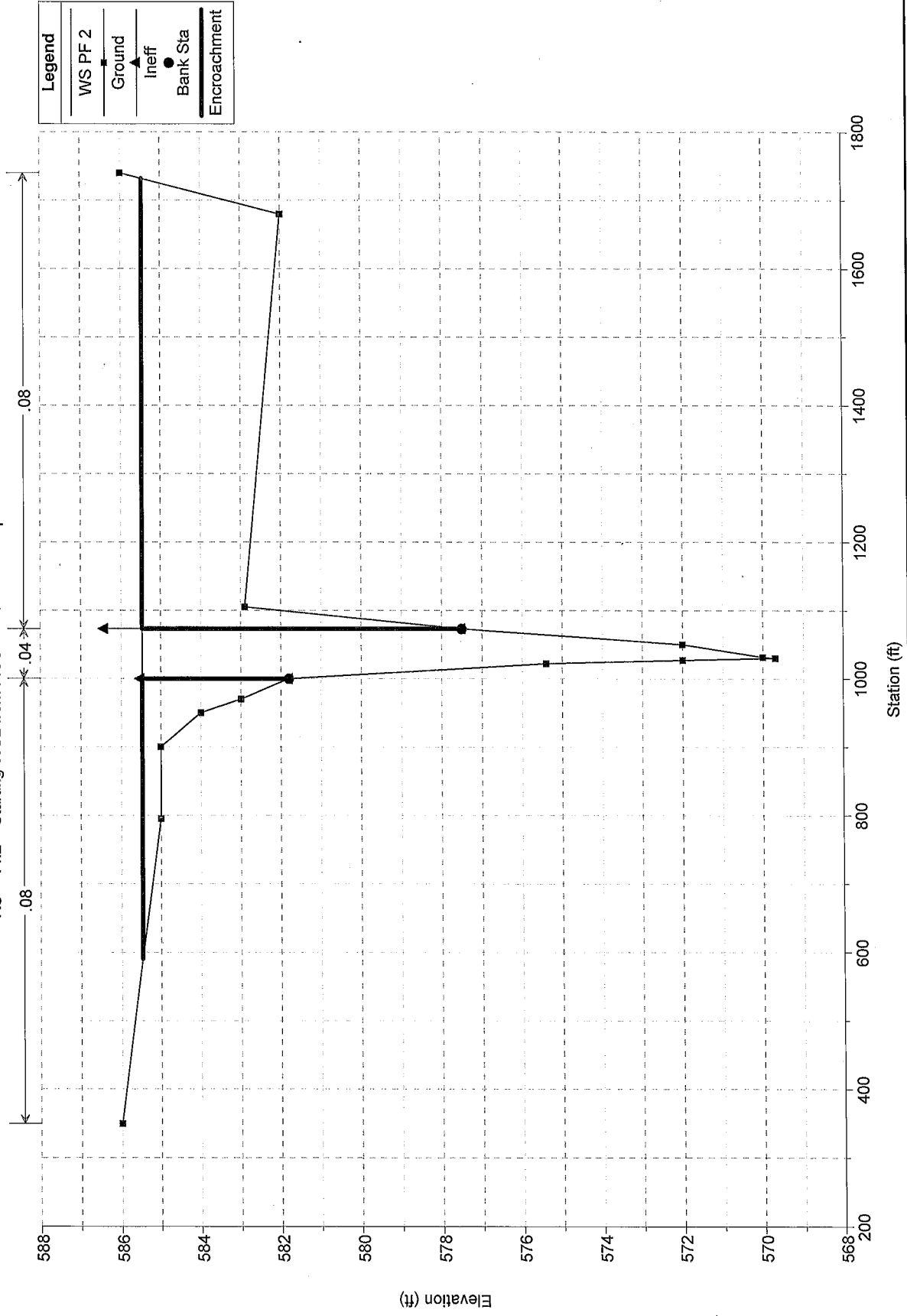
RS = 17.3



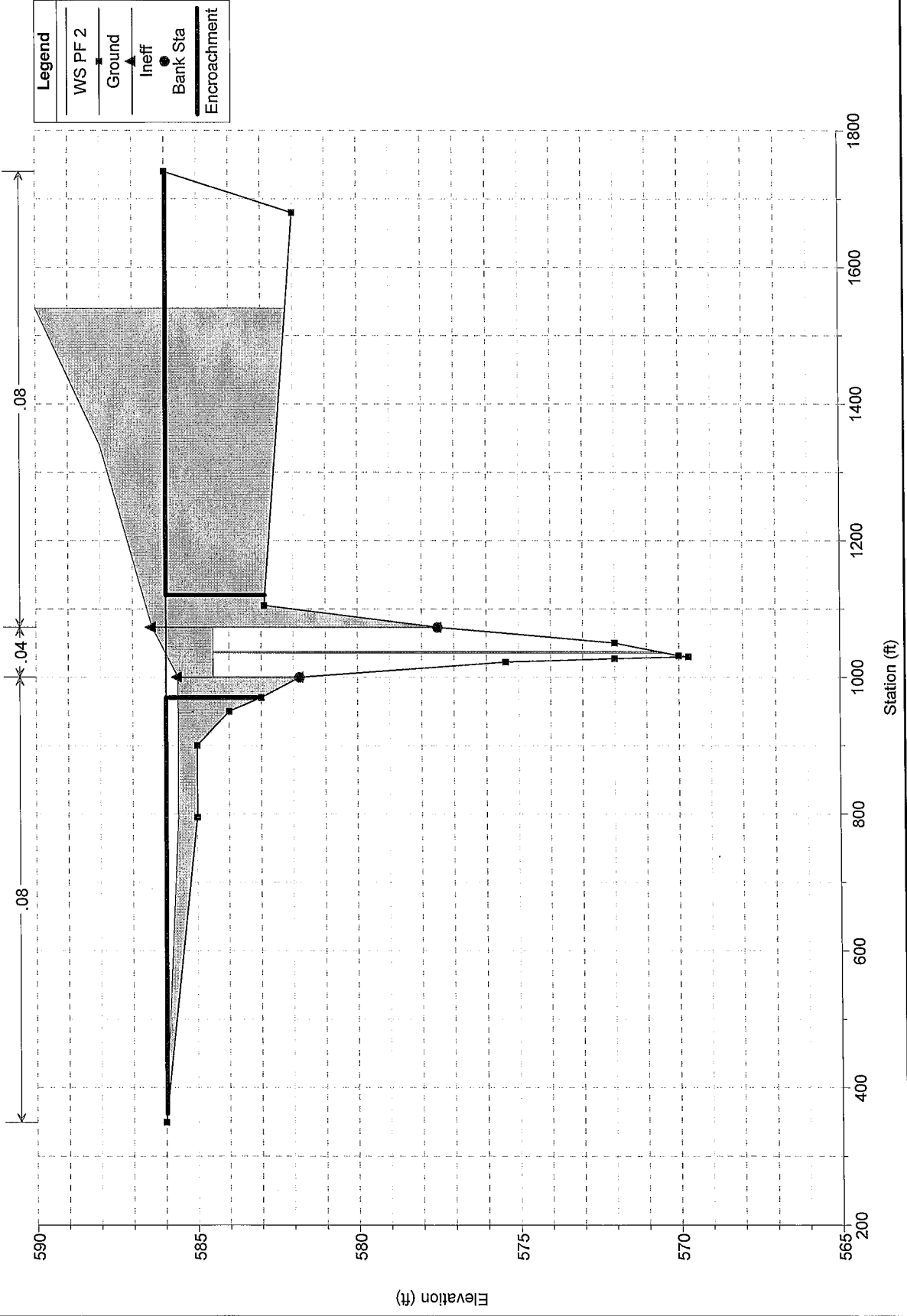
Mill Creek Floodway - Revised Existing RS = 17.4 left bank elevation was changed from 588 to 583



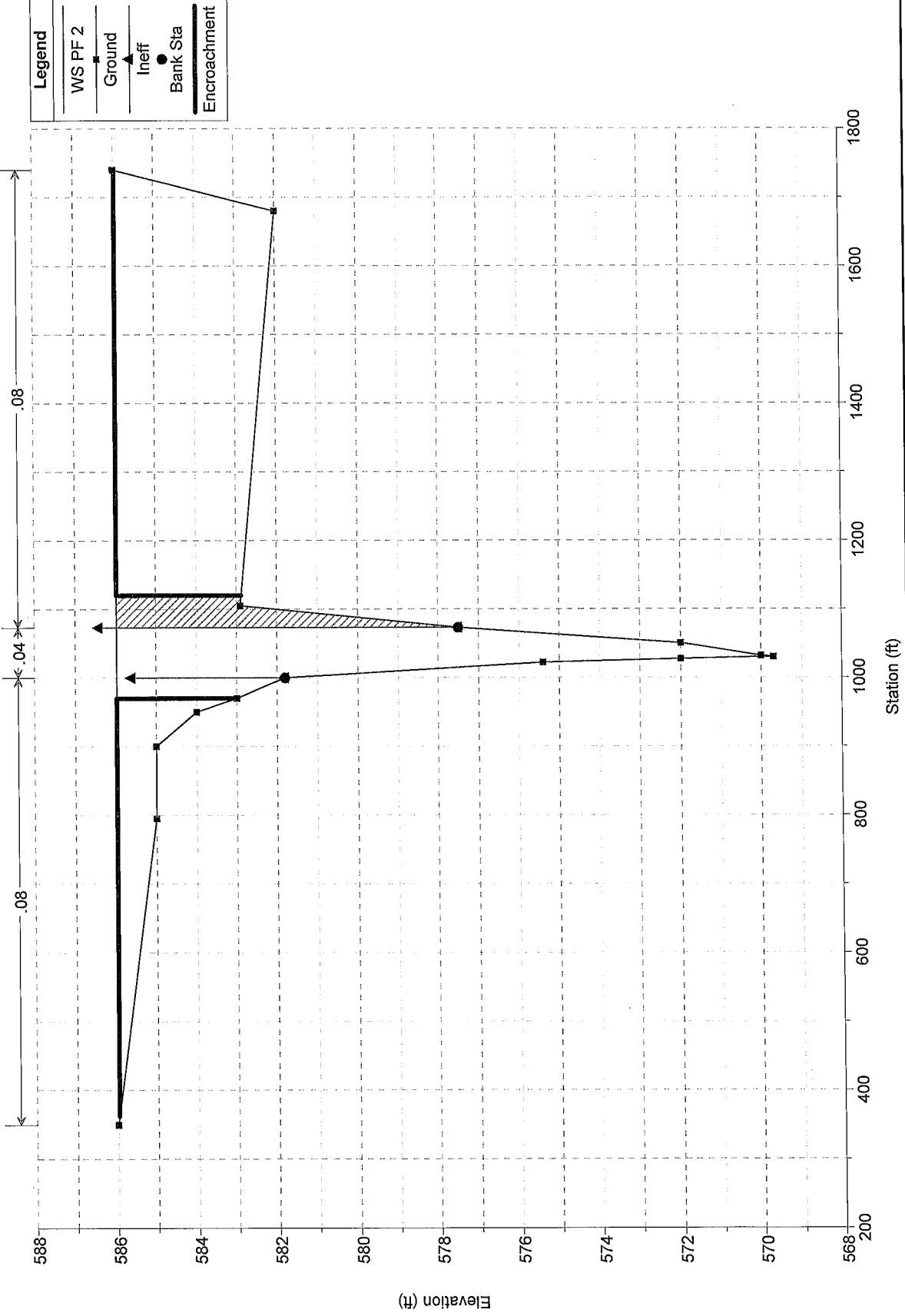
Mill Creek Floodway - Revised Proposed RS = 14.2 Starting WSL from MCSHARON.HC2 output



Mill Creek Floodway - Revised Proposed RS = 14.25 BR Bridge #1

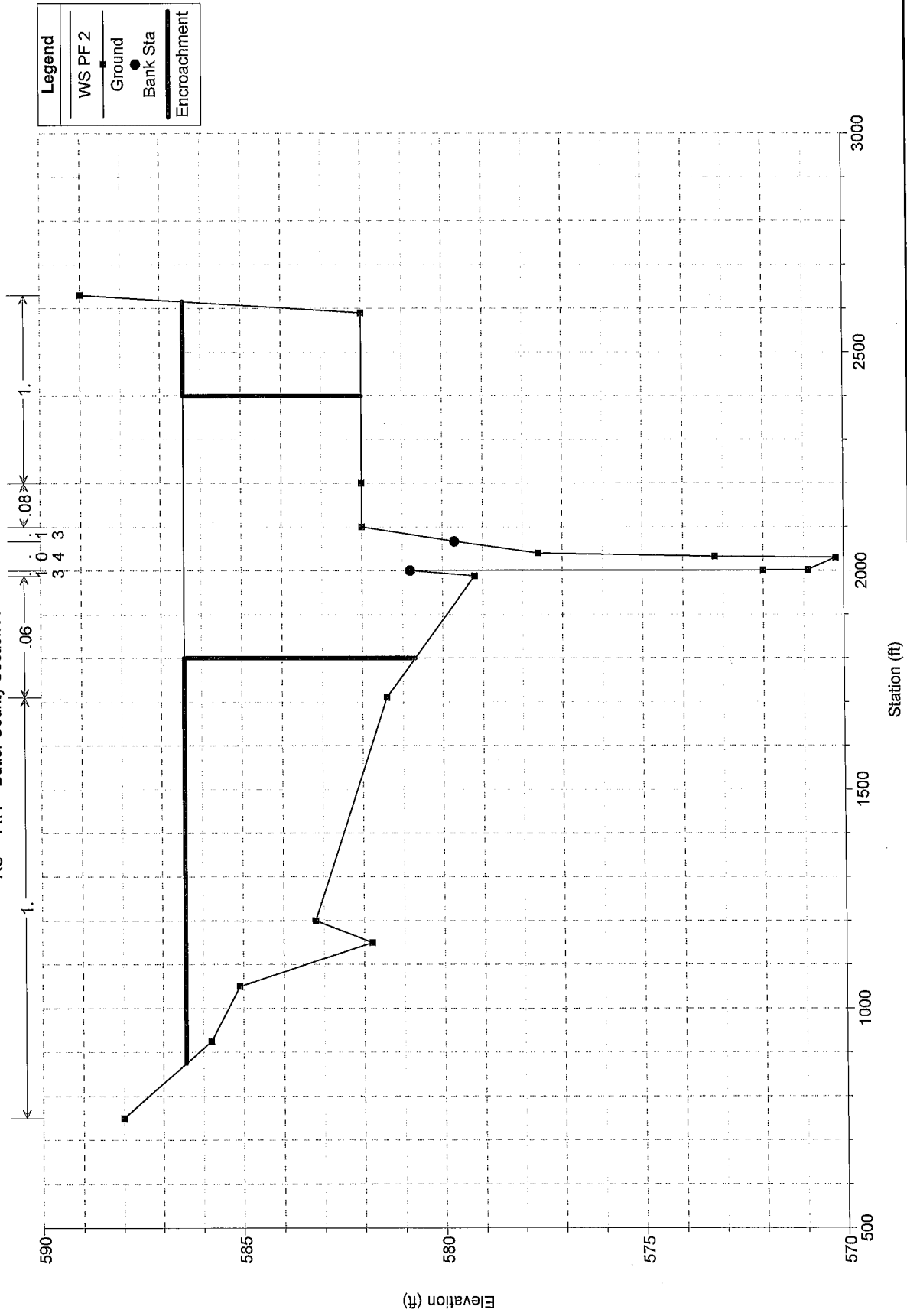


Mill Creek Floodway - Revised Proposed
RS = 14.3 This is a REPEATED section.



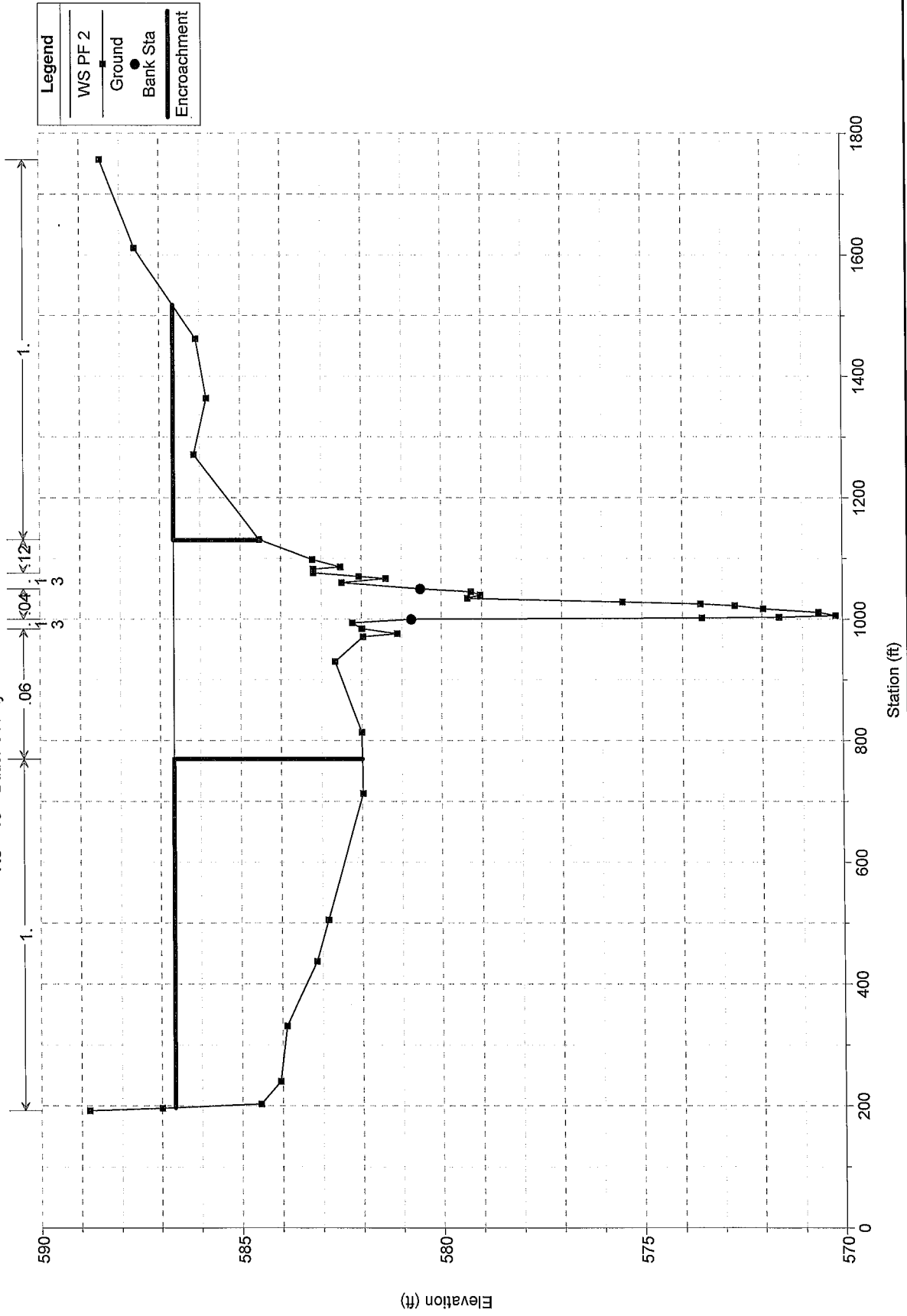
Mill Creek Floodway - Revised Proposed

RS = 14.4 Butler County Section A



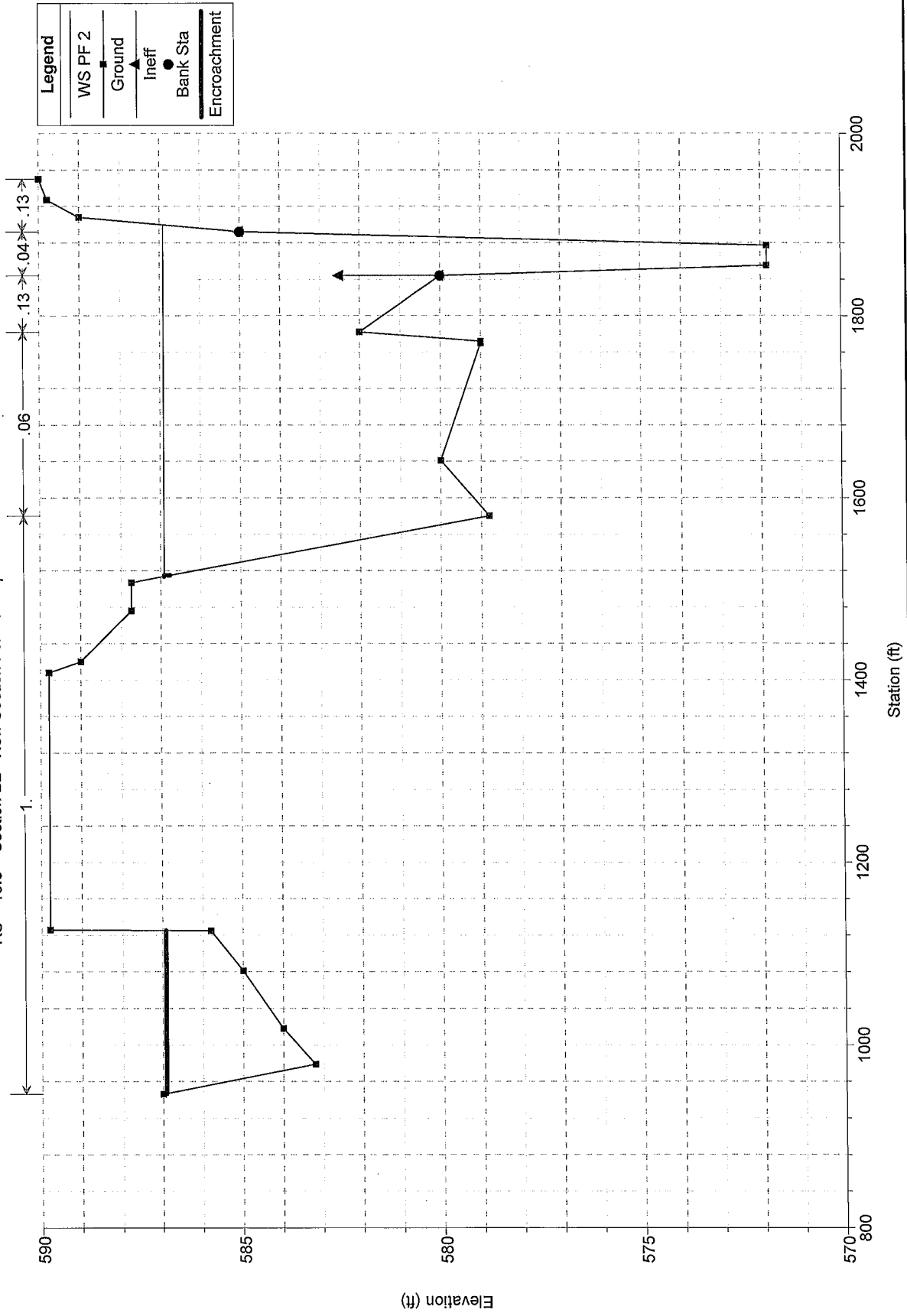
Mill Creek Floodway - Revised Proposed

RS = 15 Butler County Section B

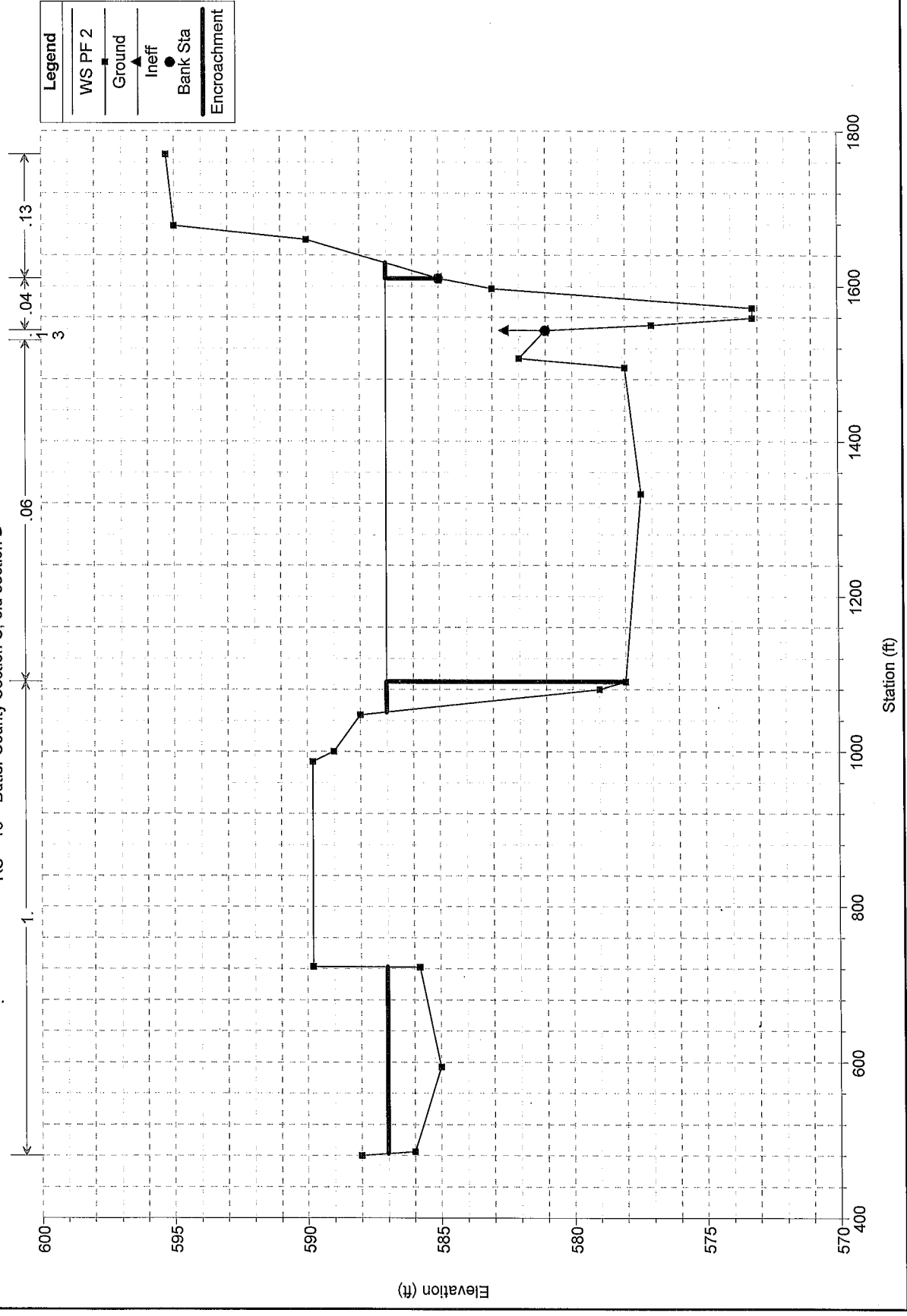


Mill Creek Floodway - Revised Proposed Section B2 - New Section For Development East of Windisch Road

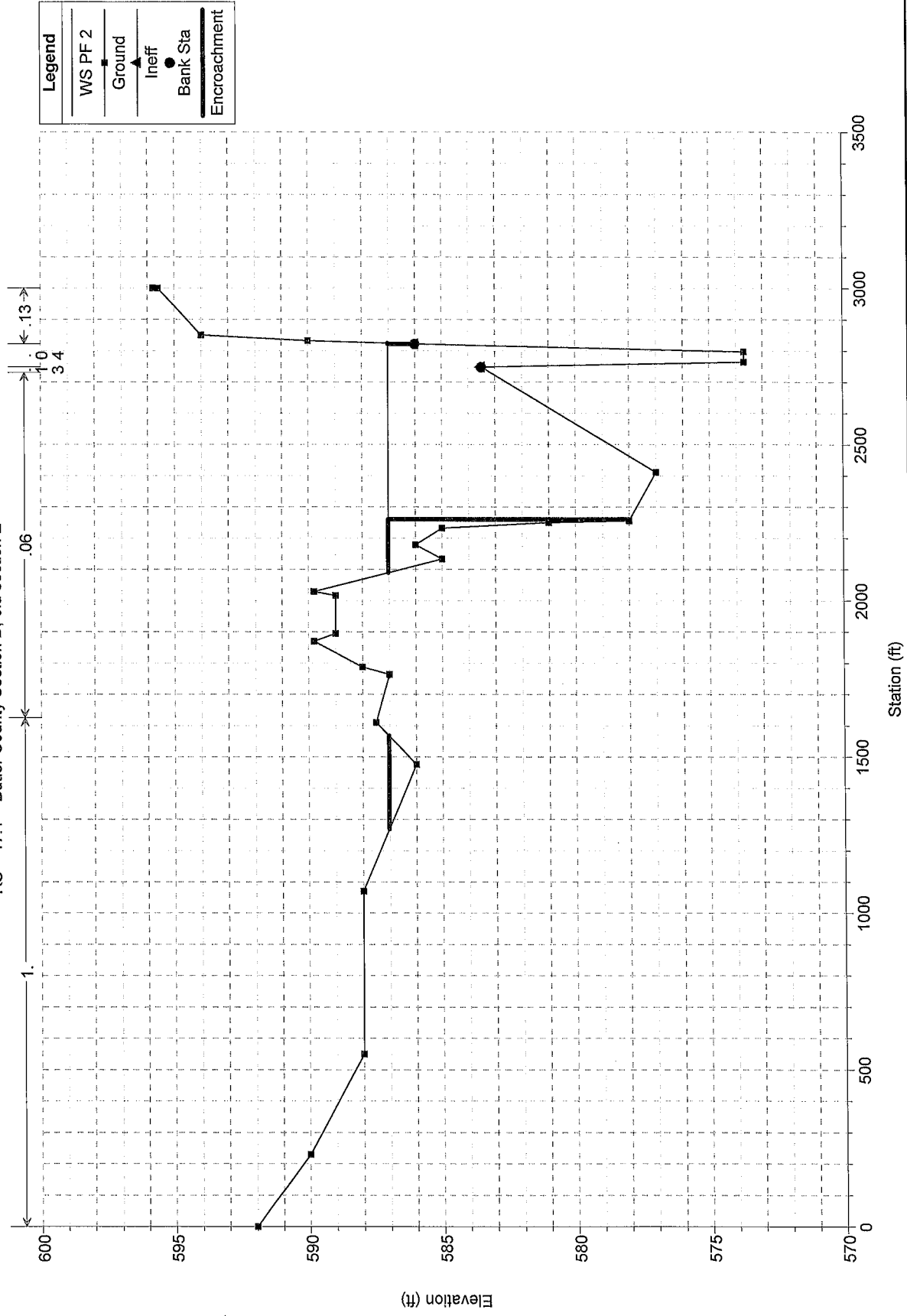
RS = 15.5



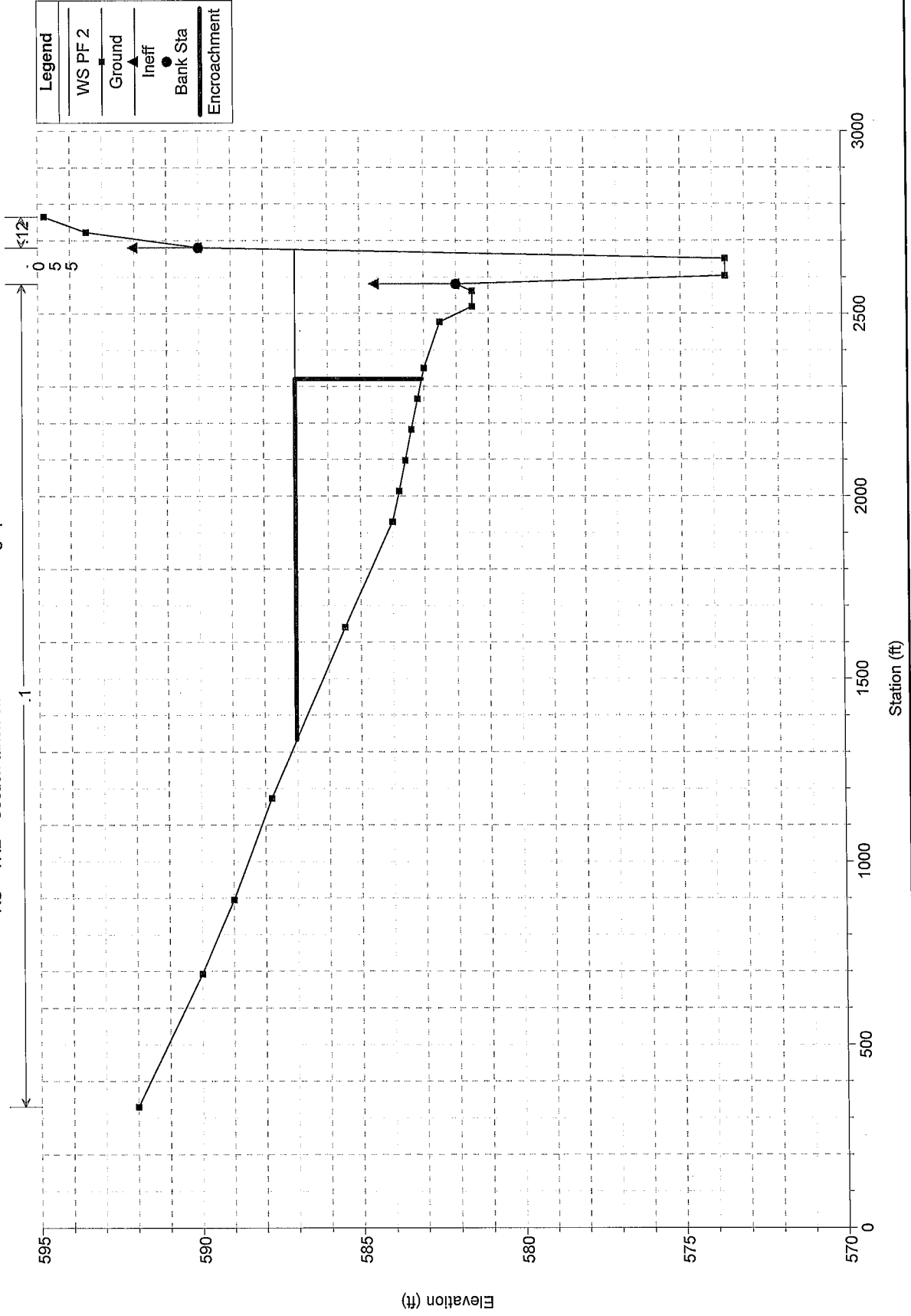
Mill Creek Floodway - Revised Proposed
 RS = 16 Butler County Section C, old section D



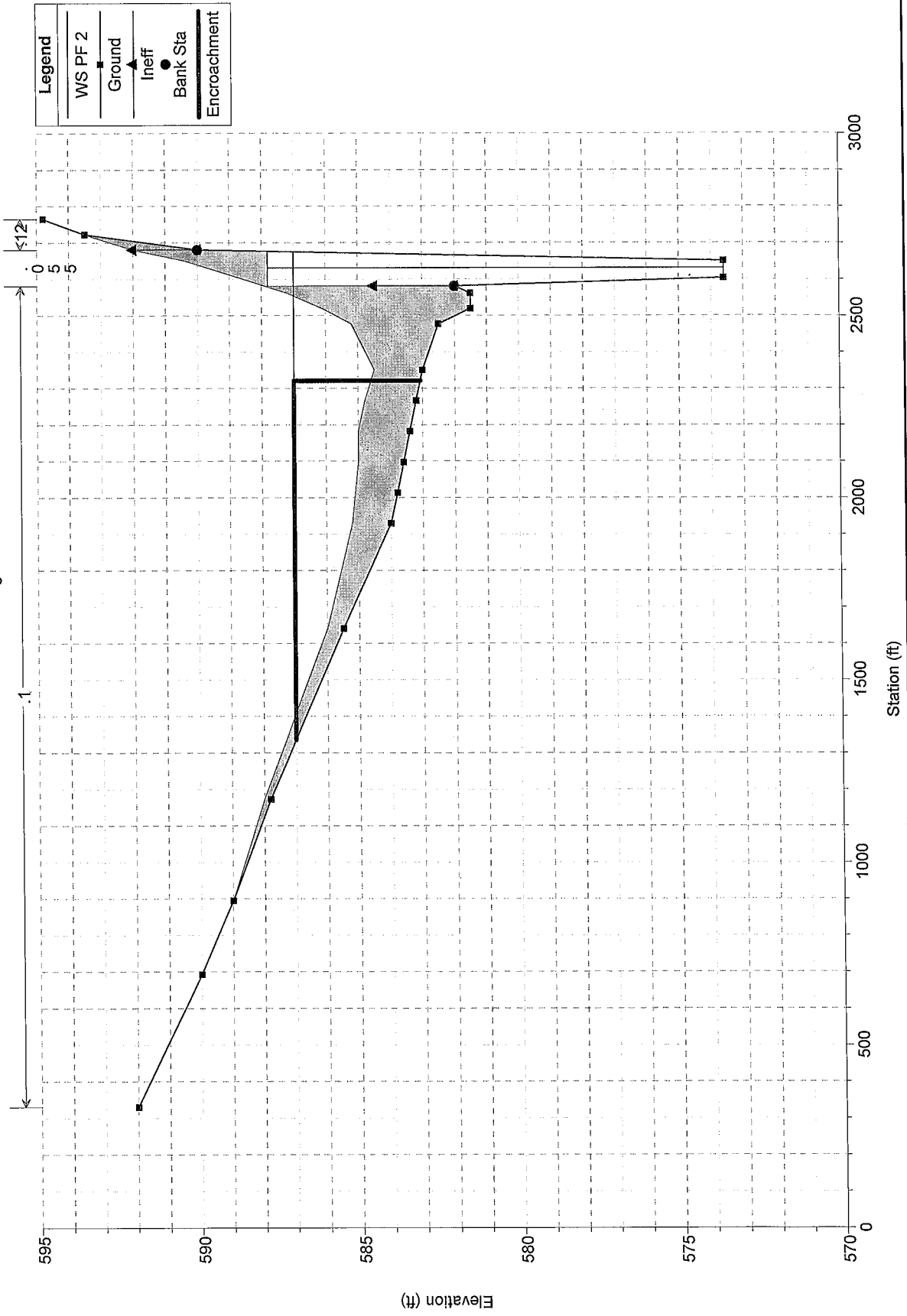
Mill Creek Floodway - Revised Proposed
 RS = 17.1 Butler County Section D, old section E



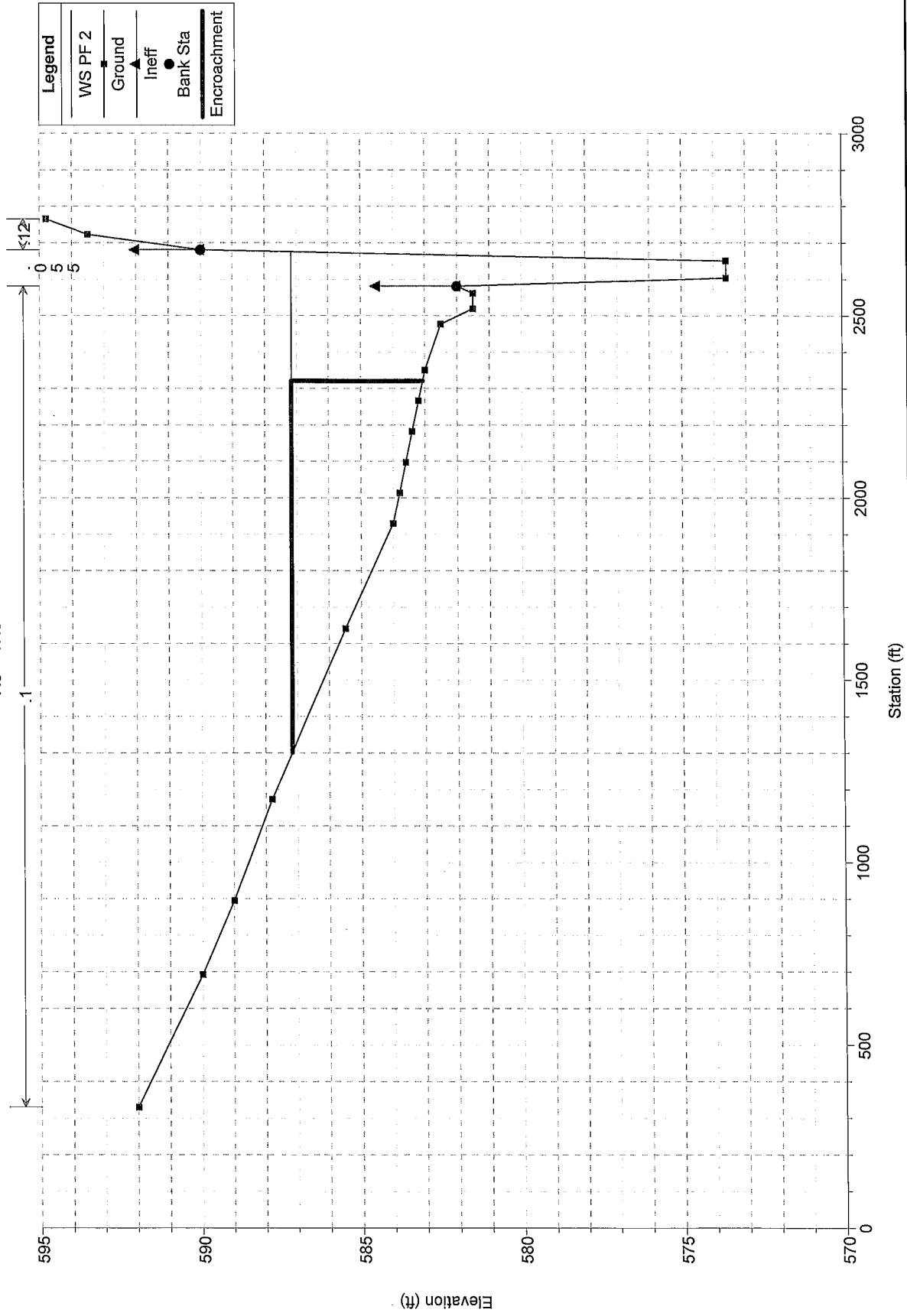
Mill Creek Floodway - Revised Proposed
 RS = 17.2 Section taken off Windisch Road bridge plan 3/84



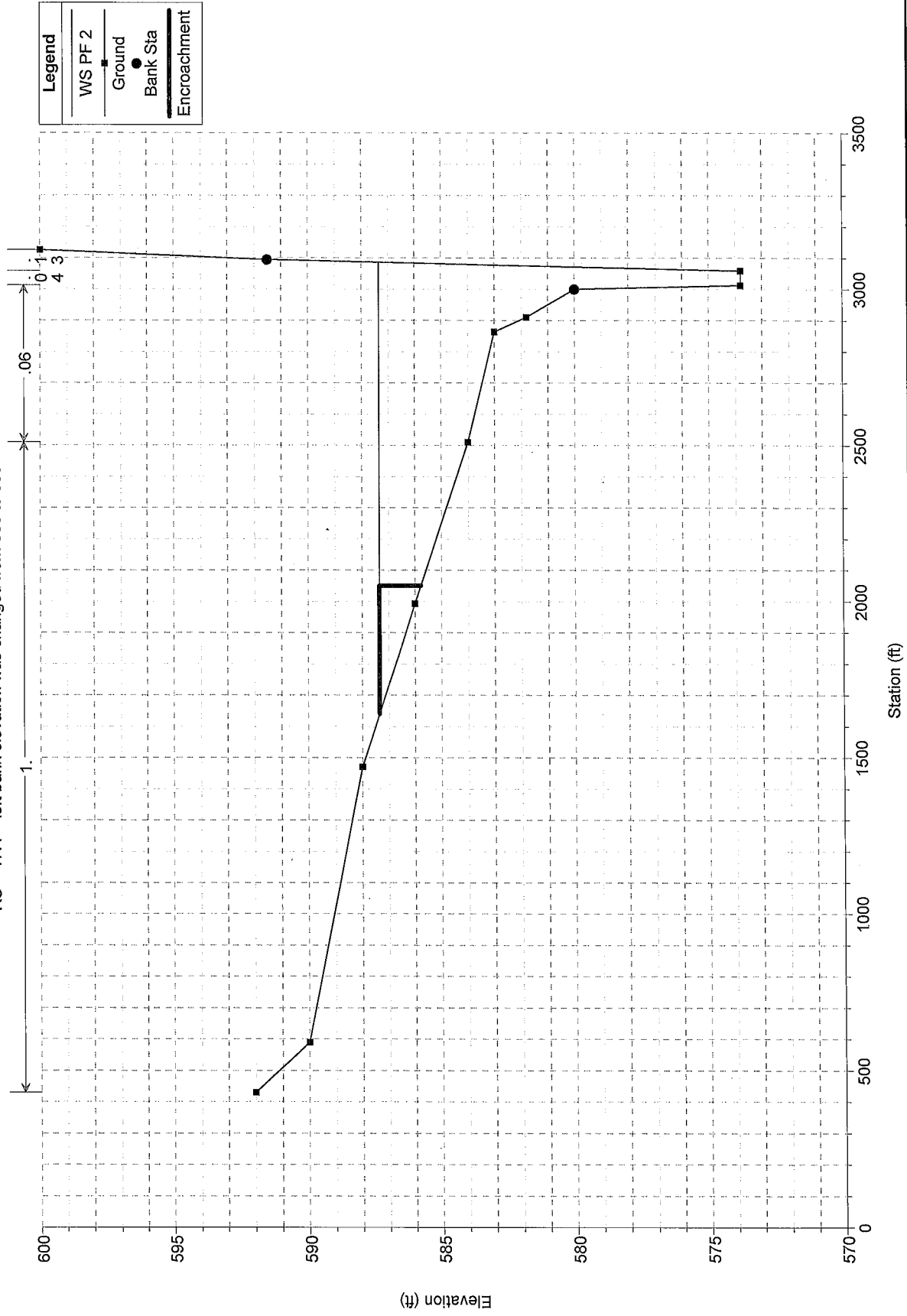
Mill Creek Floodway - Revised Proposed RS = 17.25 BR Bridge #2



Mill Creek Floodway - Revised Proposed RS = 17.3



Mill Creek Floodway - Revised Proposed RS = 17.4 left bank elevation was changed from 588 to 583





engineers • architects • planners
landscape architects • surveyors

6900 Tylersville Road, Suite A
Mason, Ohio 45040
P 513.336.6600
F 513.336.9365

www.bayerbecker.com

INTEGRITY	QUALITY	SERVICE	CONTINUOUS IMPROVEMENT
-----------	---------	---------	------------------------

07M022-000 CUT-FILL ANALYSIS

EAST BASIN EARTHWORK

Total Proposed Excavation = 78,390 C.Y. (48.59 Ac-ft)

Topsoil (12") = 30,605 C.Y. (18.97 Ac-ft)

Clay = 78,390 C.Y. - 30,605 C.Y. = 47,785 C.Y. (29.62 Ac-ft)

Flood Plain Volume Created = 78,390 C.Y.

~~WRONG~~ - (30,605 C.Y. x 25%) (Respread Topsoil)
= 70,738 C.Y. (43.85 Ac-ft) **47,785 C.Y.**

Storage Needed for Buildings 1-5 = 64,533 C.Y. (40.00 Ac-ft)

Excess volume available for Proposed = 70,738 C.Y.

- 64,533 C.Y. **NO EXCESS VOLUME**
= 6,205 C.Y.

EARTHWORK QUANTITIES

Excavation (C.Y.)	71,298 C.Y.
Embankment (C.Y.)	71,711 C.Y.

FLOOD PLAIN ANALYSIS

Flood Plain Fill = 71,711 C.Y. (Total Fill in Flood Plain) **ON-SITE**

- 16,285 C.Y. (Fill above Flood Plain Elevation, 588.40) **NOT IN FLOOD PLAIN !!! COMP**
= 55,426 C.Y. ✓

Flood Plain Storage Required = 55,426 C.Y. (Flood Plain Fill)

+ 10,648 C.Y. (Detention Volume)

+ 3,346 C.Y. (Water Quality Volume)

= **69,420 C.Y.**

Compensation Volume Provided = 71,298 C.Y. (Cut in Flood Plain)

+ 6,205 C.Y. (Excess Volume from East Basin-Above) **NO EXCESS**
~~= 77,503 C.Y.~~

71,298 CY - 69,420 CY = 1,878 CY EXCESS

User Name: Justin Elam
Project: 07M022-001 Windisch Road
Prismoidal Volume Results

Date: 10-22-07
Time: 11:56:44
Page: 1

Prismoidal Volume Results

=====

Original Surface Model: (01c020)ORIGINAL
Final Surface Model: Grading 10-22-07
Cut Compaction Factor: 0.00
Fill Compaction Factor: 0.00

Raw Cut Volume: 71298.02 cu yd
Compacted Cut Volume: 0.00 cu yd
Total Cut Volume: 71298.02 cu yd

Raw Fill Volume: 71710.84 cu yd
Compacted Fill Volume: 0.00 cu yd
Total Fill Volume: 71710.84 cu yd

Prismoidal Volume Results

=====

Original Surface Model: Constant Elevation: 588.40
Final Surface Model: Grading 10-22-07
Cut Compaction Factor: 0.00
Fill Compaction Factor: 0.00

Raw Cut Volume: 163175.59 cu yd
Compacted Cut Volume: 0.00 cu yd
Total Cut Volume: 163175.59 cu yd

Raw Fill Volume: 16285.84 cu yd
Compacted Fill Volume: 0.00 cu yd
Total Fill Volume: 16285.84 cu yd

Water Quality, Volume & Elev.

06M079-000

$$WQ_v = C \times P \times A / 12$$

WALTER
WINDISCH PROPERTY

→ DOES IT GET THERE?

$$A = 41.1 \text{ Ac (onsite)} + 14.2 \text{ Ac (offsite)} = 55.3$$

* MOUND BETWEEN THE
TWO PROPERTIES

$$P = 0.45$$

$$C = 0.8 \text{ (Industrial + Commercial)}$$

$$WQ_v = 2.765 \text{ Ac} \cdot \text{ft}$$

$$0.45 WQ_v = 2.074 \text{ Ac} \cdot \text{ft} \quad (90343 \text{ ft}^3)$$

$$\therefore Q = \frac{90343 \text{ ft}^3}{24 \text{ hr}} = 1.05 \text{ cfs}$$

$$\text{Normal Pool} = 576.94$$

$$> h = 0.84 \text{ ft}$$

$$\text{Water Quality Achieved} = 577.78$$

~~Orifice Size~~

$$Q = C_d A \sqrt{2gh}$$

$$Q = 1.05 \text{ cfs}$$

$$C_d = 0.61$$

$$g = 32.2 \text{ ft/sec}^2$$

$$h = A_{\text{pool}} h = 0.84 / 2 = 0.42 \text{ ft}$$

$$A = 0.331$$

$$\Rightarrow 7.79'' = D$$

~~7 1/2" Orifice~~

Weir

$$Q = CLH^{3/2}$$

$$Q = 1.05 \text{ cfs}$$

$$C = 3$$

$$H = 0.42 \text{ ft}$$

USE
0.84'

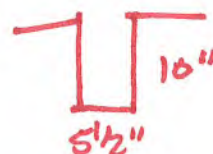
$$L = 1.29 \Rightarrow 1' 1/4'' \checkmark$$

$$L = 0.4546'$$

USE 0.45 OR

5 1/2"

WEIR



DATE 4-17-98

BAYER & BECKER ENGINEERS

CIVIL ENGINEERS • SURVEYORS • PLANNERS

PROJECT _____

BY Dms

CK'D. _____

PAGE NUMBER 1 of 3SUBJECT Union Commerce Park - Regional Detention

Criteria - Detain the difference between the 10yr pre developed and 50 yr post developed.

IS THIS A REAL FACTOR

OFFSITE DUE TO BERM ON NORTH SIDE OF BLDG'S? NB.

Area = 14.2 acres

$$\text{Slope} = \frac{596-589}{1700} = 0.0041 \approx 0.4\%$$

Average C = 77 (see pg A)Time of Concentration = 1.4 hrs. (See pg B) $Q_{10} = 11 \text{ cfs}$ $Q_{50} = 17 \text{ cfs.}$

Soil Types

FA	10%	"A"
RvB ₂	30%	"B"
SH	60%	"C"

Union Commerce Park

Area = 41.1 acres

$$\text{Slope} = \frac{590-582}{2400} = 0.0033 \approx 0.33\%$$

Present Conditionsaverage C = 70 (see pg E)Soil Type Sh - 80% Ec - 20% $\Rightarrow$ Type "C"Time of Concentration = 1.58 hrs. (see pg E) $Q_{10} = 23 \text{ cfs.}$ Future Conditionsaverage C = 91 (see pg H)Time of Concentration = 0.44 hrs (see pg I) $Q_{10} = 120 \text{ cfs}$ $Q_{50} = 159 \text{ cfs.}$

DATE 4-17-98BY Dms

CK'D. _____

BAYER & BECKER ENGINEERS

CIVIL ENGINEERS • SURVEYORS • PLANNERS

PROJECT Union CommerceParkPAGE NUMBER 2 of 3

SUBJECT _____

$$\text{Allowable Discharge} = Q_{10}(\text{site}) + Q_{10}(\text{offsite})$$

$$Q_A = 23 \text{ cfs} + 11 \text{ cfs}$$

$$Q_A = 34 \text{ cfs.}$$

$$\text{Peak Inflow} = Q_{50}(\text{site}) + Q_{50}(\text{offsite})$$

$$Q_p = 159 \text{ cfs} + 17 \text{ cfs}$$

$$Q_p = 176 \text{ cfs.}$$

$$\text{Required Volume for the entire site} = \underline{6.6 \text{ A}\cdot\text{ft}} \quad (\text{see pg L})$$

$$\text{Volume Provided} = 6.89 \text{ A}\cdot\text{ft} @ \text{elev} = 581.36 \text{ ft.} \quad (\text{see pg M})$$

Outlet Pipe

CULVERTS

RESULTS

DIAMETER (IN)	?	30	HEADWATER (FT)	FLOWRATE (CFS)
LENGTH (FT)	?	162	=====	=====
FRICTION COEFF (FT ^{1/6})	?	.018	579.50	OC 19.53
ENT+EXIT COEFF	?	1	580.00	OC 24.13
INLET CONTROL COEFF	?	.58	580.50	OC 27.98
			581.00	OC 31.36
			581.50	OC 34.42
INV ELEV OUT (FT)	?	576.05	582.00	OC 37.22
INV ELEV IN (FT)	?	577	582.50	OC 39.83
TAILWATER ELEV (FT)	?	576.05	583.00	OC 42.27
ELEV INCREMENT (FT)	?	.5	583.50	OC 44.58
			584.00	OC 46.78
			584.50	OC 48.88
			585.00	OC 50.89
			585.50	OC 52.83
			586.00	OC 54.69
			586.50	OC 56.50
			587.00	OC 58.25

ROUTE UP WEIR AND
CULVERT TOGETHER

DETAIL OUTLET
STRUCTURE

DATE 4-17-98

BY DMS

CK'D. _____

SUBJECT _____

BAYER & BECKER ENGINEERS

CIVIL ENGINEERS • SURVEYORS • PLANNERS

PROJECT _____

Union Commerce Park

PAGE NUMBER 3 of 3

Spillway Calculations.

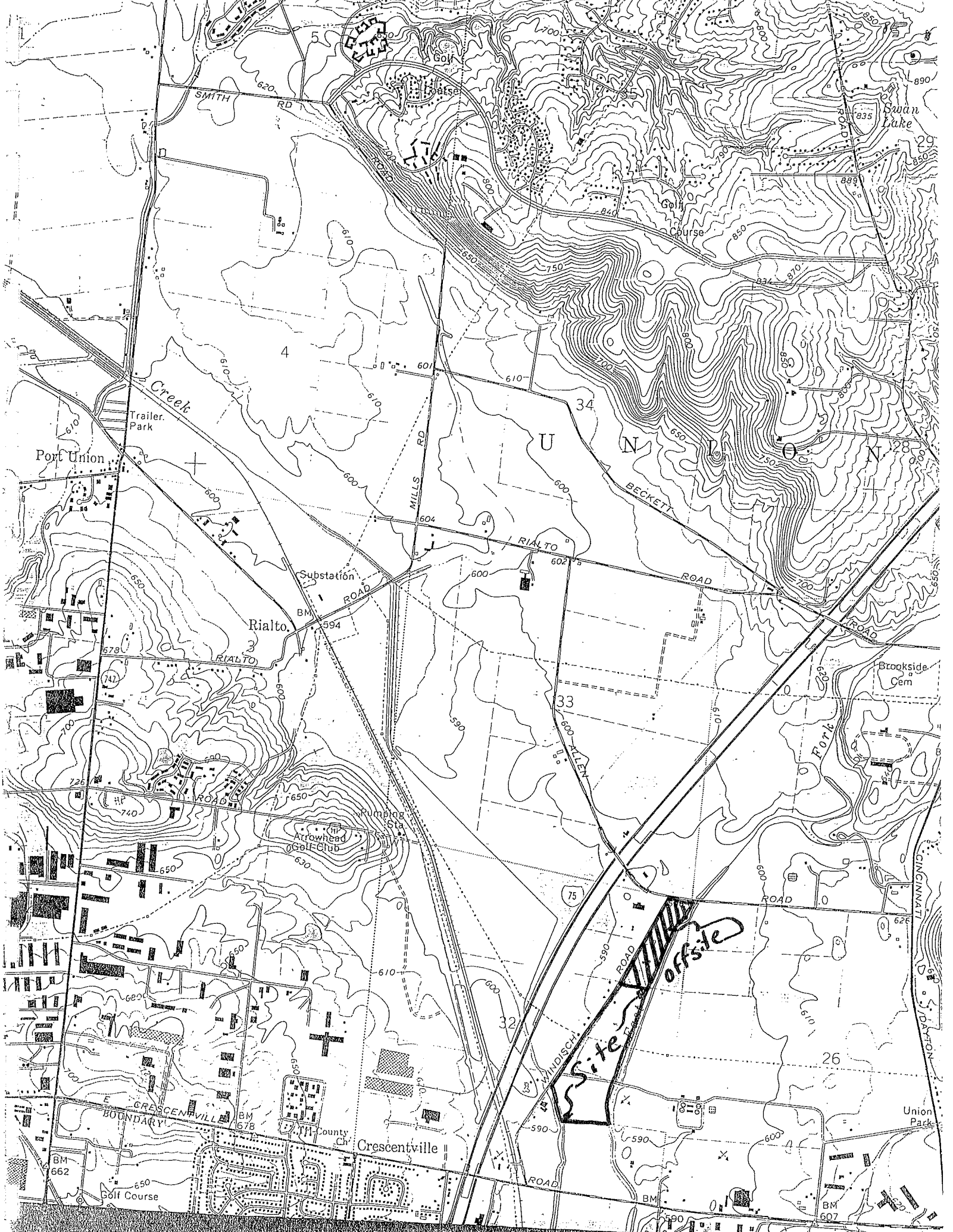
$$Q = 3.33 L H^{3/2}$$

$$L = 65 \quad H = 1.2$$

$$Q = 3.33 (65) (1.2)^{1.5}$$

$$Q = 284.5 \text{ cfs} \quad \checkmark$$

FROM WHERE?
ELEVATION? ...



Project : Windisch Rd Offsite
 County :
 S btitle: 10 year
 Subarea : off

State:

User:
 Checked: _____

Date: 03-31-97
 Date: _____

COVER DESCRIPTION	Hydrologic Soil Group			
	A	B	C	D
	Acres (CN)			
OTHER AGRICULTURAL LANDS				
Farmsteads	-----	1.42(59)	4.26(74)	8.52(82)
				-
Total Area (by Hydrologic Soil Group)	1.42	4.26	8.52	
	====	====	====	

Subarea: off TOTAL DRAINAGE AREA: 14.2 Acres WEIGHTED CURVE NUMBER: 77

Project : Windisch Rd Offsite
 County :
 Subtitle: 10 year

State:

User:
 Checked: _____

Date: 03-31-97
 Date: _____

Flow Type	2 year rain	Length (ft)	Slope (ft/ft)	Surface code	Area (sq/ft)	Wp (ft)	Velocity (ft/sec)	Time (hr)
Sheet	3.2	300	.004	f				1.090
Shallow Concent'd		1000	0.004	u				0.272
Open Channel		400					2.5	0.044
Time of Concentration = 1.41*								=====

--- Sheet Flow Surface Codes ---

A Smooth Surface	F Grass, Dense
B Fallow (No Res.)	G Grass, Burmuda
C Cultivated < 20 % Res.	H Woods, Light
D Cultivated > 20 % Res.	I Woods, Dense
E Grass-Range, Short	

--- Shallow Concentrated ---
 --- Surface Codes ---
 P Paved
 U Unpaved

- Generated for use by TABULAR method

Project : Windisch Rd Offsite

User:

Date: 03-31-97

County :

State:

Checked: _____

Date: _____

S btitle: 10 year

Total watershed area: 0.022 sq mi Rainfall type: II Frequency: 10 years

----- Subareas -----

	off
Area(sq mi)	0.02*
Rainfall(in)	4.2
Curve number	77*
Runoff(in)	1.93
Tc (hrs)	1.41*
(Used)	1.50
TimeToOutlet	0.00
Ia/P	0.14

Time (hr)	Total Flow	Subarea Contribution to Total Flow (cfs)
		off

1.0	0	0
1.3	0	0
1.6	1	1
1.9	1	1
2.0	1	1
2.1	1	1
2.2	1	1
2.3	2	2
2.4	3	3
2.5	4	4
2.6	6	6
2.7	7	7
2.8	9	9
3.0	10	10
3.2	11P	11P
3.4	10	10
3.6	8	8
3.8	7	7
4.0	6	6
4.3	4	4
4.6	3	3
5.0	3	3
5.5	2	2
6.0	2	2
6.5	1	1
7.0	1	1
7.5	1	1
8.0	1	1
9.0	1	1
20.0	1	1
22.0	1	1
26.0	0	0

P - Peak Flow

* - value(s) provided from TR-55 system routines

Project : Windisch Rd Offsite

County :

Subtitle: 50 year

State:

User:

Checked: _____

Date: 03-31-97

Date: _____

Total watershed area: 0.022 sq mi Rainfall type: II Frequency: 50 years
 ----- Subareas -----

off
 Area(sq mi) 0.02*
 Rainfall(in) 5.2
 Curve number 77*
 Runoff(in) 2.79
 Tc (hrs) 1.41*
 (Used) 1.50
 TimeToOutlet 0.00
 Ia/P 0.11

Time Total ----- Subarea Contribution to Total Flow (cfs) -----
 (hr) Flow off

1.0	0	0
1.3	1	1
1.6	1	1
1.9	1	1
2.0	1	1
2.1	2	2
2.2	2	2
2.3	3	3
2.4	5	5
2.5	7	7
2.6	9	9
2.7	11	11
2.8	13	13
3.0	15	15
3.2	17P	17P
3.4	14	14
3.6	12	12
3.8	10	10
4.0	8	8
4.3	6	6
4.6	5	5
5.0	4	4
5.5	3	3
6.0	2	2
6.5	2	2
7.0	2	2
7.5	1	1
8.0	1	1
8.5	1	1
9.0	1	1
10.0	1	1
12.0	1	1
15.0	1	1
20.0	1	1
25.0	1	1
30.0	0	0

P - Peak Flow

* - value(s) provided from TR-55 system routines

TR-55 CURVE NUMBER COMPUTATION

VERSION 1.11

Project : Windisch Rd Site
 County :
 Subtitle: pre 10
 Subarea : site

State:

User:
 Checked: _____

Date: 03-31-97
 Date: _____

COVER DESCRIPTION

Hydrologic Soil Group

A B C D
 Acres (CN)

OTHER AGRICULTURAL LANDS

Brush - brush, weed, grass mix fair

- - 41.1(70) -

Total Area (by Hydrologic Soil Group)

41.1
 =====

UBAREA: site TOTAL DRAINAGE AREA: 41.1 Acres

WEIGHTED CURVE NUMBER: 70

Project : Windisch Rd Site

County :

Subtitle: pre 10

State:

User:

Checked: _____

Date: 03-31-97

Date: _____

Flow Type	2 year rain	Length (ft)	Slope (ft/ft)	Surface code	n	Area (sq/ft)	Wp (ft)	Velocity (ft/sec)	Time (hr)
Sheet	3.2	300	.0033	f					1.178
Shallow Concent'd		1000	.0033	u					0.300
Open Channel		1100							0.102
							3.0		
									Time of Concentration = 1.58*
									=====

--- Sheet Flow Surface Codes ---

A Smooth Surface	F Grass, Dense
B Fallow (No Res.)	G Grass, Burmuda
C Cultivated < 20 % Res.	H Woods, Light
D Cultivated > 20 % Res.	I Woods, Dense
E Grass-Range, Short	

--- Shallow Concentrated ---
 --- Surface Codes ---
 P Paved
 U Unpaved

- Generated for use by TABULAR method

Project : Windisch Rd Site

County :

Subtitle: pre 10

State:

User:

Checked: _____

Date: 03-31-97

Date: _____

Total watershed area: 0.064 sq mi Rainfall type: II Frequency: 10 years
----- Subareas -----

Area(sq mi)	0.06*
Rainfall(in)	4.2
Curve number	70*
Runoff(in)	1.43
Tc (hrs)	1.58*
(Used)	1.50
TimeToOutlet	0.00
Ia/P	0.21

Time (hr)	Total Flow	Subarea Contribution to Total Flow (cfs)
		site
1.0	0	0
1.3	0	0
1.6	1	1
1.9	1	1
2.0	1	1
2.1	1	1
2.2	2	2
2.3	3	3
2.4	5	5
2.5	7	7
2.6	10	10
2.7	13	13
2.8	16	16
3.0	20	20
3.2	23P	23P
3.4	20	20
3.6	18	18
3.8	15	15
4.0	12	12
4.3	9	9
4.6	8	8
5.0	6	6
5.5	5	5
6.0	4	4
6.5	3	3
7.0	3	3
7.5	3	3
8.0	2	2
9.0	2	2
10.0	2	2
12.0	1	1
14.0	0	0

P - Peak Flow

* - value(s) provided from TR-55 system routines

TR-55 CURVE NUMBER COMPUTATION

VERSION 1.11

Project : Windisch Rd Site
 County :
 Subtitle: post10
 Subarea : site

State:

User:
 Checked: _____

Date: 03-31-97
 Date: _____

COVER DESCRIPTION

Hydrologic Soil Group
 A B C D
 Acres (CN)

FULLY DEVELOPED URBAN AREAS (Veg Estab.)
 Urban Districts
 Industrial
 Avg % imperv
 72

- - 41.1(91) -

Total Area (by Hydrologic Soil Group)

41.1
 ===

SUBAREA: site TOTAL DRAINAGE AREA: 41.1 Acres WEIGHTED CURVE NUMBER:91

Project : Windisch Rd Site
 County :
 Subtitle: post10

State:

User:
 Checked: _____

Date: 03-31-97
 Date: _____

----- Subarea #1 - site -----									
Flow Type	2 year rain	Length (ft)	Slope (ft/ft)	Surface code	n	Area (sq/ft)	Wp (ft)	Velocity (ft/sec)	Time (hr)
Sheet	3.2	300	.0033	a					0.100
Shallow Concent'd		1000	.0033	p					0.238
Open Channel		1100							0.102
								3.0	
Time of Concentration = 0.44*									=====

--- Sheet Flow Surface Codes ---

A Smooth Surface	F Grass, Dense
B Fallow (No Res.)	G Grass, Burmuda
C Cultivated < 20 % Res.	H Woods, Light
D Cultivated > 20 % Res.	I Woods, Dense
E Grass-Range, Short	

--- Shallow Concentrated ---
 --- Surface Codes ---
 P Paved
 U Unpaved

- Generated for use by TABULAR method

TR-55 TABULAR DISCHARGE METHOD

VERSION 1.11

Project : Windisch Rd Site

County :

Subtitle: post10

State:

User:

Checked: _____

Date: 03-31-97

Date: _____

Total watershed area: 0.064 sq mi Rainfall type: II Frequency: 10 years

----- Subareas -----
 Area(sq mi) site 0.06*
 Rainfall(in) 4.2
 Curve number 91*
 Runoff(in) 3.16
 Tc (hrs) 0.44*
 (Used) 0.40
 TimeToOutlet 0.00
 Ia/P 0.05
 (Used) 0.10

Time (hr)	Total Flow	site	Subarea Contribution to Total Flow (cfs)
11.0	4	4	
11.3	5	5	
11.6	7	7	
11.9	16	16	
12.0	29	29	
12.1	55	55	
12.2	95	95	
12.3	120P	120P	
12.4	117	117	
12.5	87	87	
12.6	60	60	
12.7	44	44	
12.8	33	33	
13.0	21	21	
13.2	16	16	
13.4	13	13	
13.6	11	11	
13.8	10	10	
14.0	9	9	
14.3	8	8	
14.6	7	7	
15.0	6	6	
15.5	6	6	
16.0	5	5	
16.5	4	4	
17.0	4	4	
17.5	4	4	
18.0	4	4	
19.0	3	3	
20.0	3	3	
21.0	2	2	
22.0	0	0	

Peak Flow

* - value(s) provided from TR-55 system routines

Project : Windisch Rd Site

County :

Subtitle: post50

State:

User:

Checked: _____

Date: 03-31-97

Date: _____

Total watershed area: 0.064 sq mi Rainfall type: II Frequency: 50 years
 ----- Subareas -----

	site
Area(sq mi)	0.06*
Rainfall(in)	5.2
Curve number	91*
Runoff(in)	4.18
Tc (hrs)	0.44*
(Used)	0.40
TimeToOutlet	0.00
Ia/P	0.04
(Used)	0.10

Time (hr)	Total Flow	Subarea Contribution to Total Flow (cfs)
		site
11.0	5	5
11.3	7	7
11.6	10	10
11.9	21	21
12.0	38	38
12.1	73	73
12.2	126	126
12.3	159P	159P
12.4	154	154
12.5	116	116
12.6	80	80
12.7	58	58
12.8	44	44
13.0	28	28
13.2	21	21
13.4	17	17
13.6	15	15
13.8	13	13
14.0	12	12
14.3	10	10
14.6	9	9
15.0	8	8
15.5	8	8
16.0	7	7
16.5	6	6
17.0	6	6
17.5	5	5
18.0	5	5
19.0	4	4
20.0	4	4
21.0	3	3
26.0	0	0

Peak Flow

* - value(s) provided from TR-55 system routines

Project :

County :

Subtitle:

State:

User:

Checked: _____

Date: 04-17-98

Date: _____

Drainage Area: 41.1 Acres

Rainfall Frequency: 50 years

Rainfall-Type: II

Runoff: 4.2 inches

Peak Inflow: 176 cfs

Peak Outflow: 34 cfs

Detention Basin Storage Volume: 1.94 inches or 6.6 acre feet

Union Commerce Center
Union Township
Butler County, Ohio
4/17/98

CALCULATED 04-17-1998 11:42:10
DISK FILE: c:\pond2\NORTHPAR.VOL

Planimeter scale: 1 inch = 50 ft.

Elevation (ft)	Planimeter (sq.in.)	Area (acres)	A1+A2+sqr(A1*A2) (acres)	* Volume (acre-ft)	Volume Sum (acre-ft)
576.94					2.07 WQV
577.00 78	19.16	1.10	0.00	0.00	0.00 STM
578.00	21.76	1.25	3.52	1.17	1.17
580.00	30.62	1.76	4.49	2.99	4.17
581.36	*I*	2.26	6.01	2.72	6.89 DET
582.00	43.80	2.51	6.37	4.25	8.41 VOL

I ---> Interpolated area from closest two planimeter readings.

* Incremental volume computed by the Conic Method for Reservoir Volumes.

STORMWATER
DETENTION
CALCULATE STORAGE THIS WAY...

577.78 UP TO 6.6 AC·FT OF STORAGE @ ELEV XX.XX'

576.94 TO 577.78 WQV NOT TO BE
INCLUDED IN DETENTION STORAGE VOLUME

